

• 2022 FORM + FUNCTION

REVISION DATE

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JUDITH A. TUCKER

STATE OF WASHINGTO.

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REMODEL SE 33RD F ER ISLAND,

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SHEET TITLE

SITE PLAN

PROJ INFO

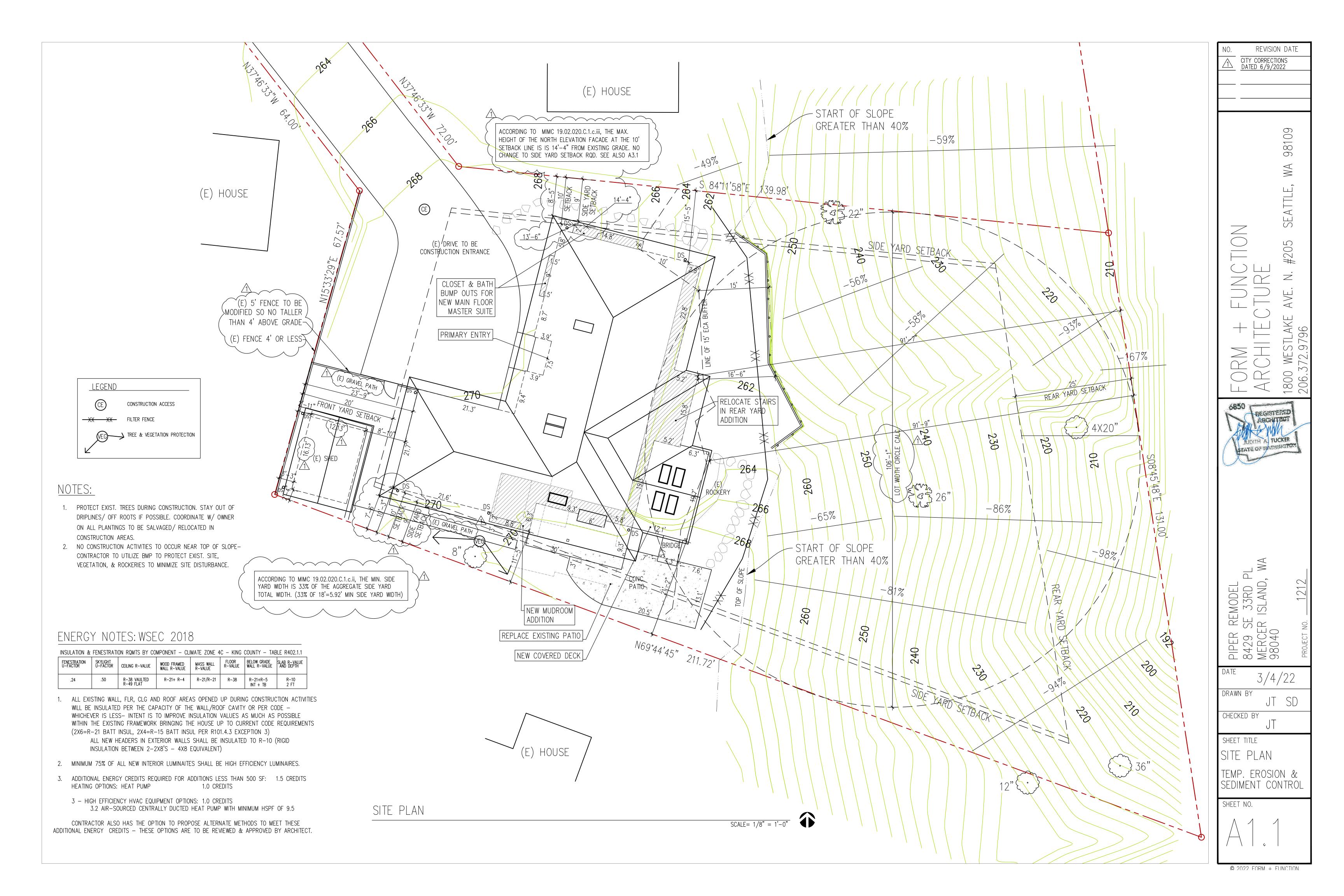
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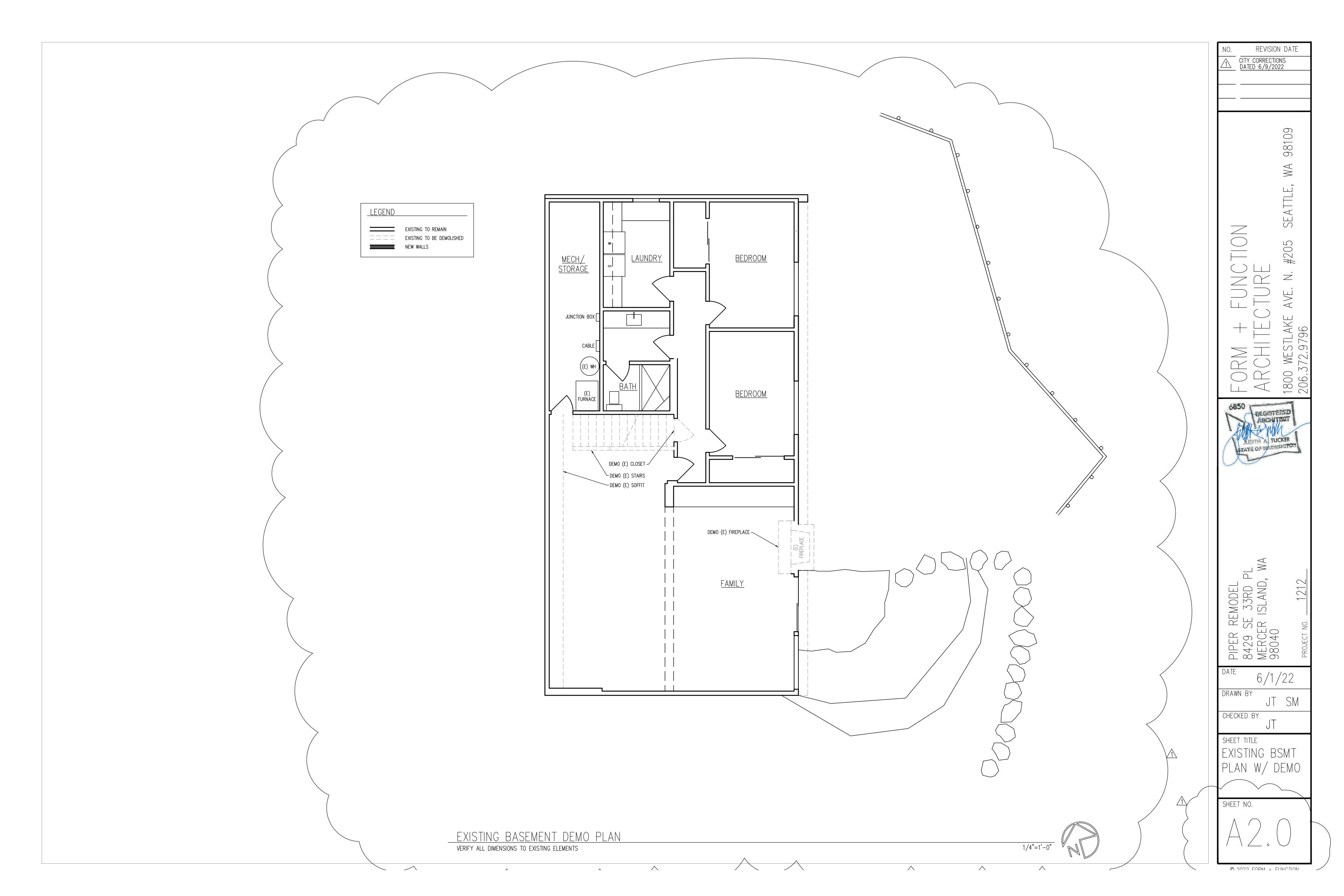
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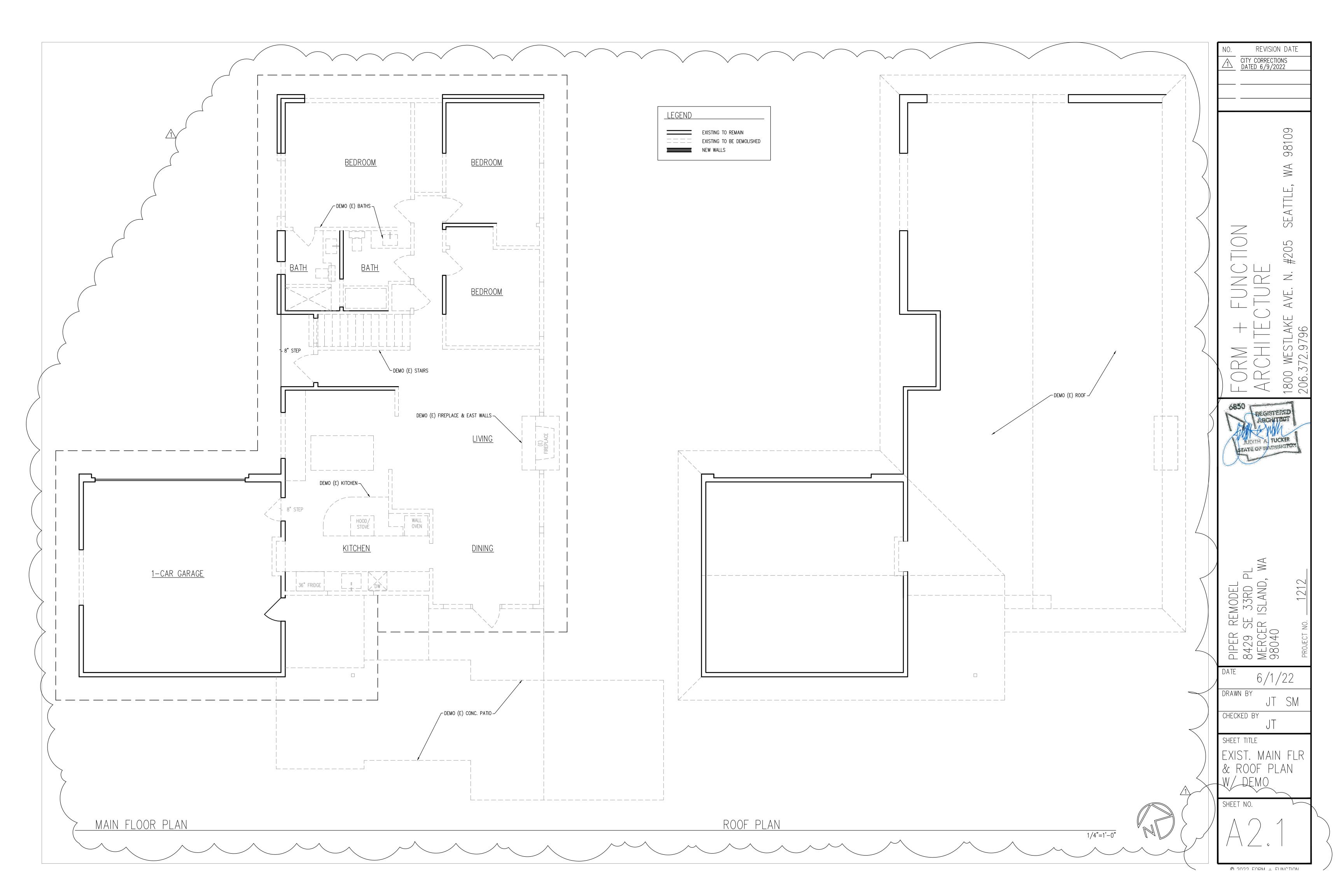
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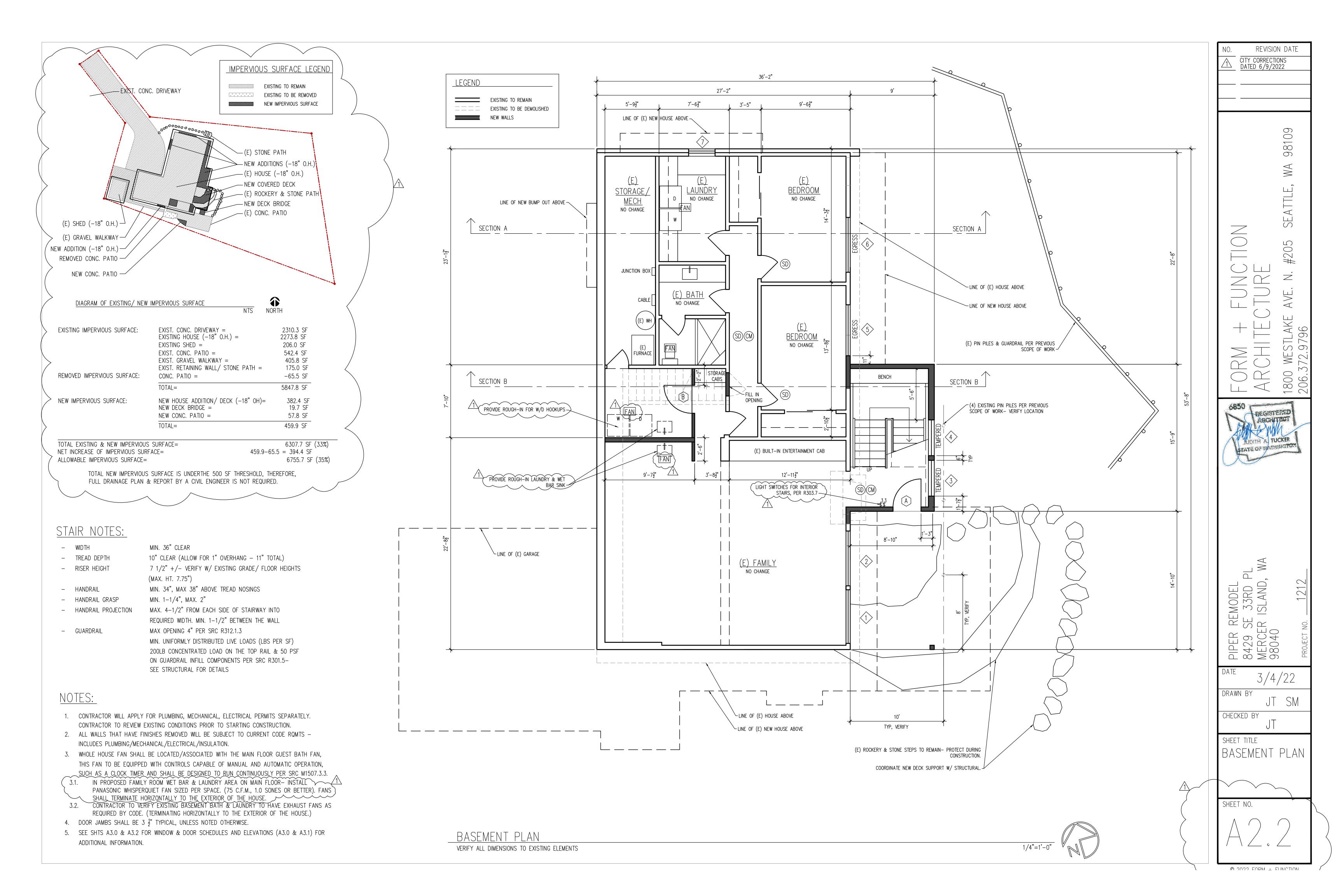
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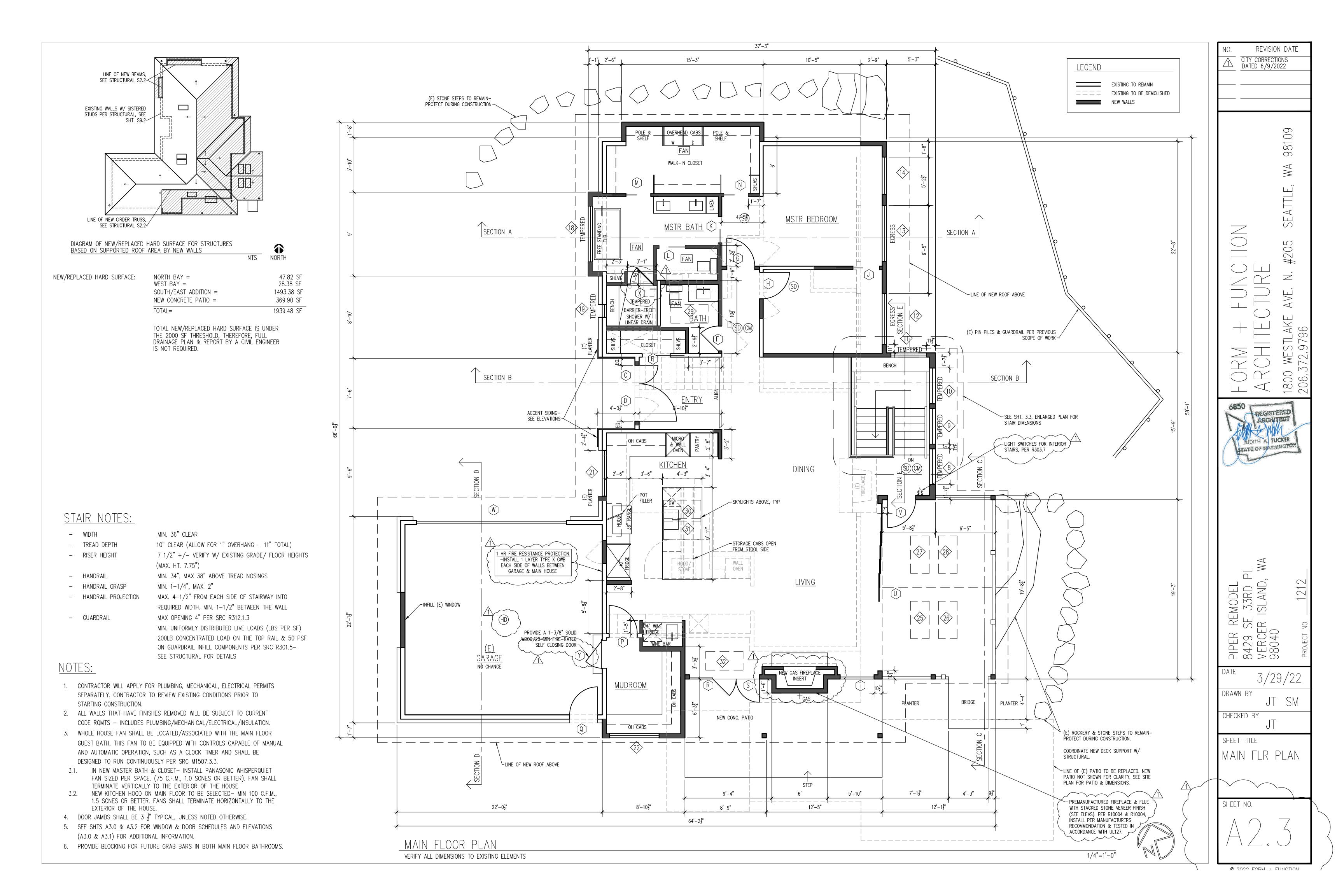
CITY CORRECTIONS DATED 6/9/2022

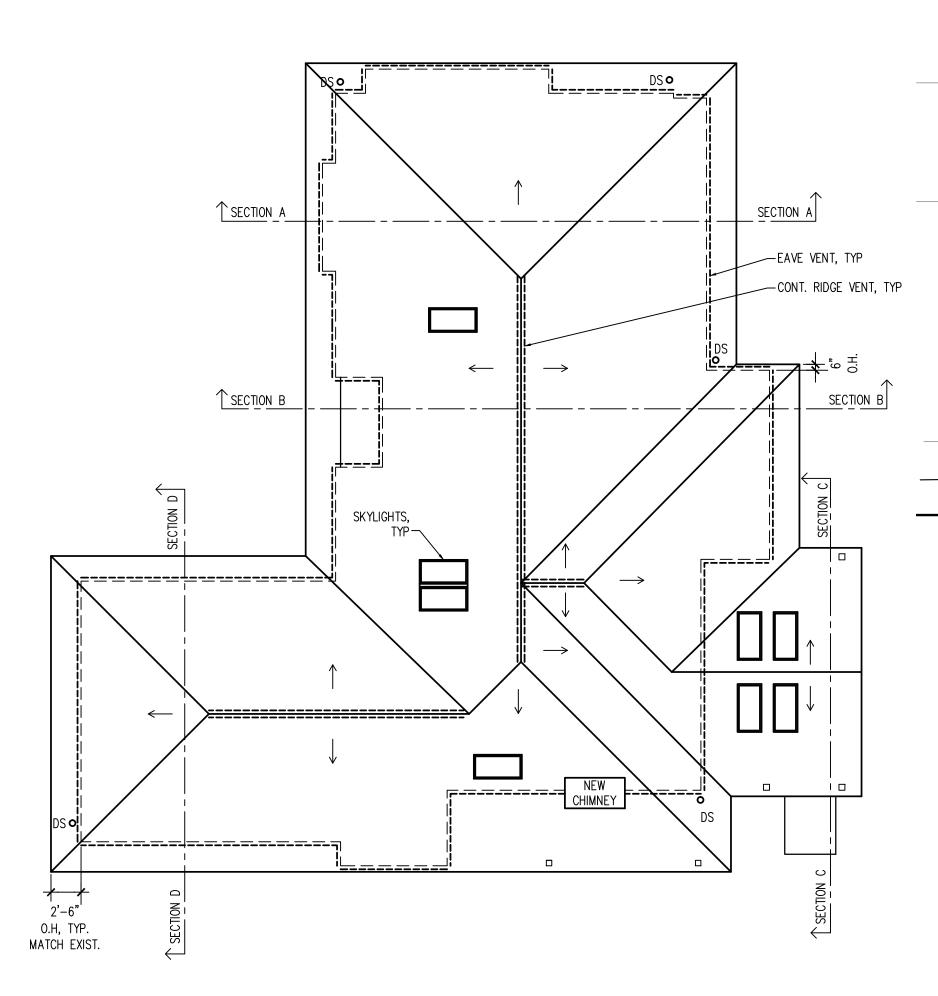












ROOF PLAN

CONNECT ALL NEW GUTTERS/ DOWNSPOUTS TO EXISTING SITE DRAINAGE SYSTEM

ROOF VENTILATION NOTES:

CONTRACTOR TO PROVIDE NEW VENTING TO MEET CODE REQUIREMENTS PER IRC R806.2 (SEE NOTES ON ROOF PLAN FOR PROPOSED VENTILATION SOLUTIONS):

1 SQ.FT. OF VENTING PER 300 SQ.FT. OF AREA TO BE VENTED (1/150 REDUCED TO 1/300 PROVIDED THAT AT LEAST 40% AND NOT MORE THAN 50% OF THE RQD VENTING PROVIDED IN THE UPPER PORTION OF THE SPACE — MIN 3' ABOVE EAVE LINE.

1" AIR SPACE REQUIRED ABOVE ROOF INSULATION

EAVE/SOFFIT VENTS — (3) 2" DIAMETER VENTS PER RAFTER BAY 9 SQ IN.

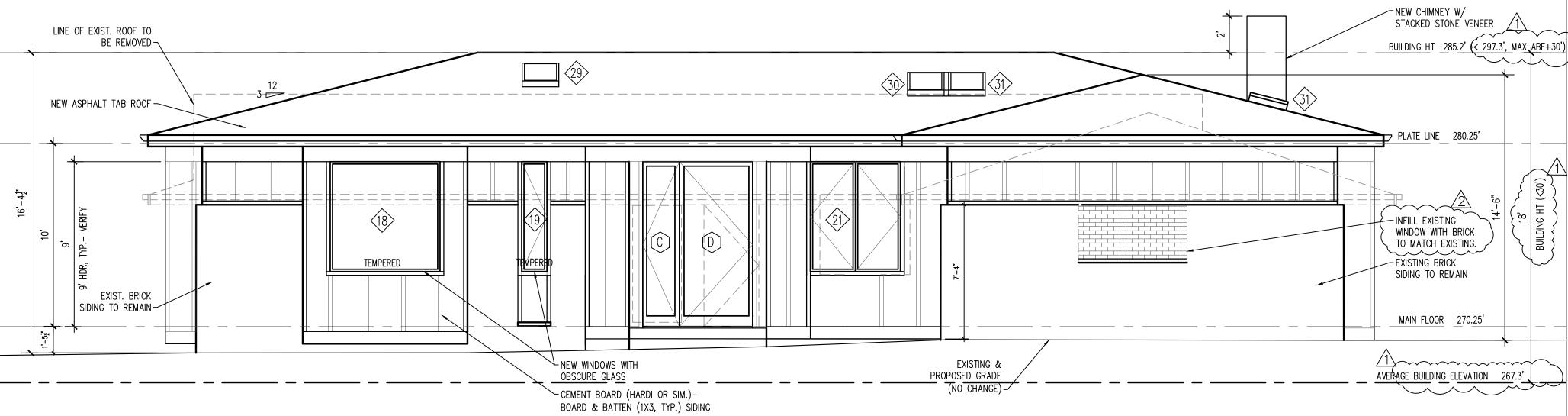
+/- PER BAY MIN.

HOUSE/GARAGE: 2396.5 SF/300= 8.0 SF (1152.0 SQ IN) RQD

REQUIRED: 576.0 SQ IN RQD (32.0 LF)
PROPOSED: 1056.5 SQ IN (58.7 LF)
SOFFIT:

REQUIRED: 576.0 SQ IN RQD (64 RAFTER BAYS)
PROPOSED: 1053.0 SQ IN (117.0 RAFTER BAYS)

PROPOSED VENTILATION MEETS/EXCEEDS CODE RQMT FOR 1/300 FOR UNIQUE SITUATIONS THAT ARISE DURING CONSTRUCTION COORDINATE VENTILATION (& INSULATION) RQMTS WITH ARCHITECT



U-VALUE NOTES

WEST ELEVATION
SEE SHT. A3.2 FOR DOOR SCHEDULE

WINDOW SIZE OPERATION MATERIAL

MINDOW SCHEDULE— BASEMENT & MAIN FLOOF

	IVIAIVIV	WIINDOW SIZE	OI LIVATION	WATLINAL	I WII GIV	GLAZING	O VALUE	INOILS	
	1	5'-9" x 4'-2"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON	
•	2	5'-10" x 4'-2"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON	
	3	3'-10" x 4'-2"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON, TEMPERED MULLED WITH #8	
•	4	3'-10" x 4'-2"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON, TEMPERED MULLED WITH #9	
	5	5'-6" x 4'-2"	CSMT	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.28 MIN	ARGON, EGRESS	
	6	5'-6" x 4'-2"	CSMT	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.28 MIN	ARGON, EGRESS	
	7	3'-0" x 2'-4"	AWNING	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.28 MIN	ARGON, (E) OPENING	
	8	3'-10" x 10'-8"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON, TEMPERED MULLED WITH #3	
	9	3'-10" x 10'-8"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON, TEMPERED MULLED WITH #4	
	10	3'-10" x 10'-8"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON, TEMPERED	
	11	3'-10" x 10'-8"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON, TEMPERED	
	12	5'-6" x 6'-2"	CSMT	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.28 MIN	ARGON, EGRESS	
	13	6'-0" x 6'-2"	CSMT	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.28 MIN	ARGON, EGRESS	
	14	4'-0" x 6'-2"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON	
	15 15	3'-0" x 2'-2"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON, TRANSOM	
=	16	3'-0" x 2'-2"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON, TRANSOM	
	17	3'-0" x 2'-2"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON, TRANSOM	
	18	6'-0" x 6'-0"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, OBSCURE	0.27 MIN	ARGON, TEMPERED SANDBLASTED	
	19	1'-4" x 6'-0"	CSMT	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, OBSCURE	0.28 MIN	ARGON, TEMPERED SANDBLASTED	
	20	6'-0" x 1'-9"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON, TRANSOM	
	21	4'-11" x 6'-0"	CSMT	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.28 MIN	ARGON	
	22	5'-4" x 1'-6"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON	1.
	23	7'-6" x 1'-9"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW=E, CLR	0.27 MIN	ARGON, TRANSOM	2.
	24	3'-0" x 1'-9"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON, TRANSOM	±\\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \
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LEGEND

EXISTING TO REMAIN

EXISTING TO BE DEMOLISHED

NEW WALLS

WINDOW SCHEDULE- ROOF

MARK	WINDOW SIZE	OPERATION	MATERIAL	MFGR	GLAZING	U-VALUE	NOTES
25	2'-0" x 4'-0"	SKYLIGHT	ALUMINUM	VELUX	LOW-E, CLR	0.50	ARGON
26	2'-0" x 4'-0"	SKYLIGHT	ALUMINUM	VELUX	LOW-E, CLR	0.50	ARGON
27	2'-0" x 4'-0"	SKYLIGHT	ALUMINUM	VELUX	LOW-E, CLR	0.50	ARGON
28	2'-0" x 4'-0"	SKYLIGHT	ALUMINUM	VELUX	LOW-E, CLR	0.50	ARGON
29	2'-0" x 4'-0"	SKYLIGHT	ALUMINUM	VELUX	LOW-E, CLR	0.50	ARGON
30	2'-0" x 4'-0"	SKYLIGHT	ALUMINUM	VELUX	LOW-E, CLR	0.50	ARGON
31	2'-0" x 4'-0"	SKYLIGHT	ALUMINUM	VELUX	LOW-E, CLR	0.50	ARGON
32	2'-0" x 4'-0"	SKYLIGHT	ALUMINUM	VELUX	LOW-E, CLR	0.50	ARGON

WINDOW GENERAL NOTES:

- 1. ALL WINDOWS TO BE NFRC CERTIFIED.
- 2. CONTRACTOR TO CONFIRM ROUGH OPENING REQUIREMENT W/ MNF'R
- 3. WINDOW MFGR TO BE MARVIN SIGNATURE MODERN (VELUX FOR SKYLIGHTS). SCHEDULE ASSUMES ALUMINUM (EBONY FINISH) W/ LOW E 272 GLASS-ARGON. SUBSTITUTIONS ARE ACCEPTABLE AS LONG AS WINDOWS MEET THE ENERGY CODE REQMTS LISTED ON SHEET A1.0
- 4. ALL EXTERIOR WINDOW OPENINGS TO BE WRAPPED W/ VIDAFLEX FOR APPROVED EQUAL PEAL & STICK MEMBRANE AND METAL FLASHINGS PER NORTHWEST WALL AND CEILING BUREAU STANDARD DETAILS.
- 5. INSTALL TEMPERED/SAFETY GLAZING AS REQUIRED PER IRC R308 AND NOTED ABOVE.

FORM + FUNCTION
ARCHITECTURE
1800 WESTLAKE AVE. N. #205 S
206.372.9796

STATE OF WASHINGTON

1/4"=1'-0"

REVISION DATE

98109

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CITY CORRECTIONS DATED 6/9/2022

CLIENT REVISIONS
DATED 6/9/2022

REMODEL SE 33RD PL ER ISLAND, WA

PIPER F 8429 S MERCEF 98040

DATE 3/4/22

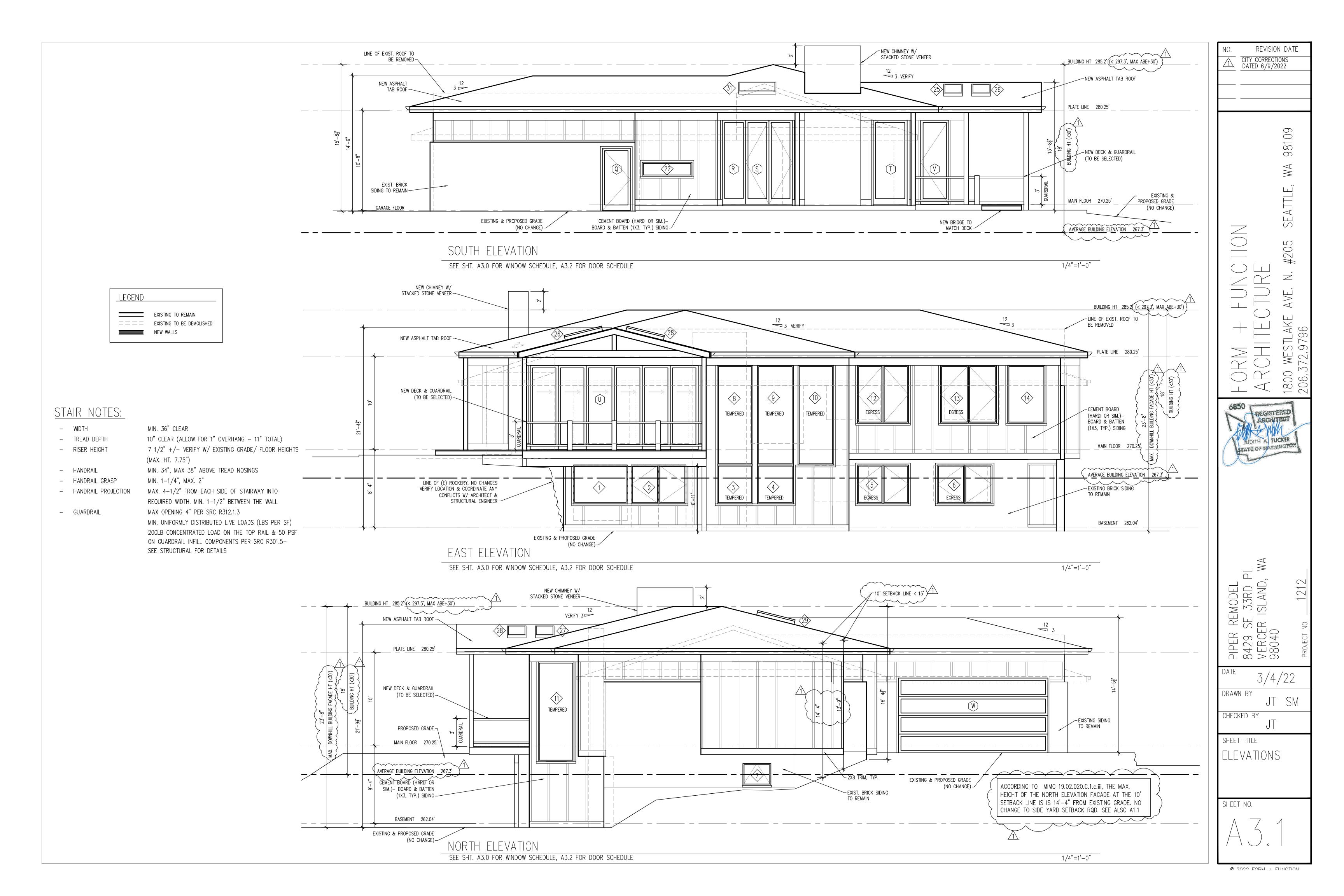
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SHEET TITLE
EXTERIOR ELEV
ROOF PLAN
WINDOW

SCHEDULE

SHEET NO.



-NEW FLAT BTM TRUSS LINE OF EXIST. ROOF TO ROOF, PER STRUCTURAL BE REMOVED -DOOR SCHEDULE- BASEMENT & MAIN FLOOR PLATE LINE 280.25' DOOR SIZE MARK OPERATION MATERIAL GLAZING U-VALUE NOTES LOW E/ARGON | 0.30 MIN | THRESHOLD BY MFGR W/ $3'-0" \times 6'-8" \mid SWING$ GLASS/ SC WOOD TEMPERED WEATHERSTRIPPING FOR TIGHT SEAL KEYED LOCK & DEADBOLT (MATCH HOUSE KEY) ←(E) FLOOR STRUCTURE TO $3'-0" \times 6'-8"$ | SWING SC WOOD NA PRIVACY LATCH REMAIN W/ NEW CANTILEVERS/ EXTENSIONS MSTR BEDROOM MSTR BATH LOW E/ARGON | 0.20 MIN | THRESHOLD BY MFGR W/ 2'-0" x 9'-0" | SWING/ SC WOOD OR FRENCH DR | GLASS / SC WOOD TEMPERED WEATHERSTRIPPING FOR TIGHT SEAL-COORD. ASTRIGAL OPTIONS W/ ARCHITECT LOW E/ARGON | 0.20 MIN | THRESHOLD BY MFGR W/ 4'-0" x 9'-0" | SWING/ SC WOOD OR ____MAIN_FLOOR___270.25' TEMPERED FRENCH DR | GLASS/ SC WOOD WEATHERSTRIPPING FOR TIGHT SEAL KEYED LOCK & DEADBOLT (MATCH HOUSE KEY) LEGEND NA $3'-0" \times 6'-8" \mid POCKET$ SC WOOD -PROVIDE R-38 INSULATION IN EXISTING TO REMAIN NEW EXPOSED FLOOR FRAMING EXISTING TO BE DEMOLISHED PER WSEC 2018 R402.1.1 HALL **BEDROOM LAUNDRY** STORAGE/ $2'-8" \times 6'-8" \mid SWING$ SC WOOD NΑ PRIVACY LATCH NEW WALLS NO CHANGE NO CHANGE NO CHANGE <u>MECH</u> -(E) SLAB ON GRADE & NO CHANGE FOUNDATION TO REMAIN $3'-0" \times 6'-8" \mid SWING$ SC WOOD PRIVACY LATCH BASEMENT 262.04' 2'-10" x 6'-8" | SWING NΑ SC WOOD PRIVACY LATCH SECTION A-A NOT USED FOR CLARITY 1/4"=1'-0" VERIFY ALL DIMENSIONS TO EXISTING ELEMENTS NEW FLAT BTM TRUSS NΑ PRIVACY LATCH 2'-10" x 6'-8" | POCKET SC WOOD NEW COVERED DECK TRUSS ROOF ROOF, PER STRUCTURAL -W/ EXPOSED T&G DECKING LID -SEE STRUCTURAL FOR DETAILS -LINE OF EXIST. ROOF TO 2'-10" x 6'-8" | POCKET NA SC WOOD PRIVACY LATCH BE REMOVED PLATE LINE 280.25' NA 2'-10" x 6'-8" | POCKET SC WOOD PRIVACY LATCH -(E) WALLS W/ ADDITIONAL HÉIGHT TO NÉW PLATES PER NA STRUCTURAL 2'-8" x 6'-8" | POCKET SC WOOD PRIVACY LATCH COVERED DECK <u>GARAGE</u> NΑ 2'-8" x 6'-8" | POCKET SC WOOD NA WOOD BRIDGE PER PLAN- ALL -(E) SLAB ON GRADE & MATERIALS IN CONTACT W/ CONCRETE **FOUNDATION TO REMAIN** TO BE PRESSURE TREATED- SEE NOT USED FOR CLARITY STRUCTURAL NOTES ¬ $3'-0" \times 6'-8"$ SWING MAIN FLOOR 270.25' LOW E/ARGON $3'-0" \times 6'-8" \mid SWING$ GLASS/ SC WOOD 0.30 MIN THRESHOLD BY MFGR W/ TEMPEŔED WEATHERSTRIPPING FOR TIGHT SEAL 2'-6" x 9'-0" GLASS/ SC WOOD LOW E/ARGON FIXED/ 0.30 MIN I THRESHOLD BY MFGR W/ (E) ROCKERY TO REMAIN, PROTECT SECTION D-D TEMPERED DURING CONSTRUCTION FRENĆH DR WEATHERSTRIPPING FOR TIGHT SEAL VERIFY LOCATION & COORDINATE ANY 1/4"=1'-0" CONFLICTS W/ ARCHITECT & $(2) 2'-6" \times 9'-0"$ GLASS/ SC WOOD SWING/ LOW E/ARGON 0.30 MIN THRESHOLD BY MFGR W/ STRUCTURAL ENGINEER -TEMPEŔED FRENCH DE WEATHERSTRIPPING FOR TIGHT SEAL BASEMENT 262.04' SECTION C-C 3'-8" x 9'-0" LOW E/ARGON FIXED/ GLASS/ SC WOOD THRESHOLD BY MFGR W/ 0.30 MIN FRENĆH DR TEMPERED WEATHERSTRIPPING FOR TIGHT SEAL 1/4"=1'-0" NEW FLAT BTM TRUSS THRESHOLD BY MFGR W/ U (6) 3'-0" x 9'-0" | SLIDER/ LOW E/ARGON | 0.30 MIN | GLASS/ SC WOOD ROOF, PER STRUCTURAL -TEMPEŔED FRENCH [WEATHERSTRIPPING FOR TIGHT SEAL LINE OF EXIST. ROOF TO LOW E/ARGON | 0.30 MIN | THRESHOLD BY MFGR W/ 3'-0" x 9'-0" | SWING/ BE REMOVED -GLASS/ SC WOOD TEMPERED FRENCH DR WEATHERSTRIPPING FOR TIGHT SEAL KEYED LOCK & DEADBOLT (MATCH HOUSE KEY) OBSCURE GLASS/ LOW E/ARGON 16'-0" x 8'-0" | GARAGE THRESHOLD BY MFGR W/ PLATE LINE 280.25' 0.30 MIN | WEATHERSTRIPPING FOR TIGHT SEAL SC WOOD TEMPERED $2'-6" \times 6'-0"$ | SHOWER GLASS NA TEMPERED GLASS EXIST. RECESSED ENTRY— REPLACE EXIST. SLATE TILE W/ NEW TO BE SELECTED. VERIFY WATERPROOFING 20 MIN RATED DR ON CLOSER ALUMINUM THRESHOLD BY PEMCO OR EQUAL <u>ENTRY</u> NΑ $3'-0" \times 6'-8" \mid SWING$ SC WOOD INTACT @ DEMOLITION OR COORDINATE SMOKE GASKETING FOR A TIGHT SEAL W/ ARCHITECT FOR UPDATED DETAIL. ~ KEYED LOCK & DEADBOLT (MATCH HOUSE KEY) DOOR GENERAL NOTES: MAIN FLOOR 270.25' 1. ALL DOORS TO BE NFRC CERTIFIED. SEE SHT A3.3 FOR TYPICAL WALL 2. CONTRACTOR TO CONFIRM ROUGH OPENING REQUIREMENT W/ MNF'R PROVIDE R-38 INSULATION IN SECTION W/ ADDITIONAL DETAIL (E) SLAB ON GRAPE & HALL 3. ALL INTERIOR & EXTERIOR DOORS BY LOEWEN OR SIMPSON OR EQUIVALENT. NEW EXPOSED FLOOR FRAMING, ' <u>BEDROOM</u> PER WSEC 2018 R402.1.17 4. SET EXTERIOR DOORS IN DOOR PAN PER NORTHWEST WALL & CLG BUREAU STANDARD DETAILS 5. ALL EXT. DOOR OPENINGS TO BE WRAPPED W/ VIDAFLEX F OR APPROVED EQUAL PEEL & STICK OR NO CHANGE METAL FLASHINGS PER THE NORTHWEST WALL & CLG BUREAU STANDARD DETAILS 6. ALL U-VALUES PROVIDED FOR DOORS ARE PRESCRIPTIVE VALUES (MINIMUMS TO BE USED) UNTIL BASEMENT 262.04' SPECIFIC MANUFACTIRERS/DOOR MODELS ARE SELECTED. 7. ALL HARDWARE TO BE LEVER TYPE- FINISH TO BE SELECTED. SECTION B-B 1/4"=1'-0"

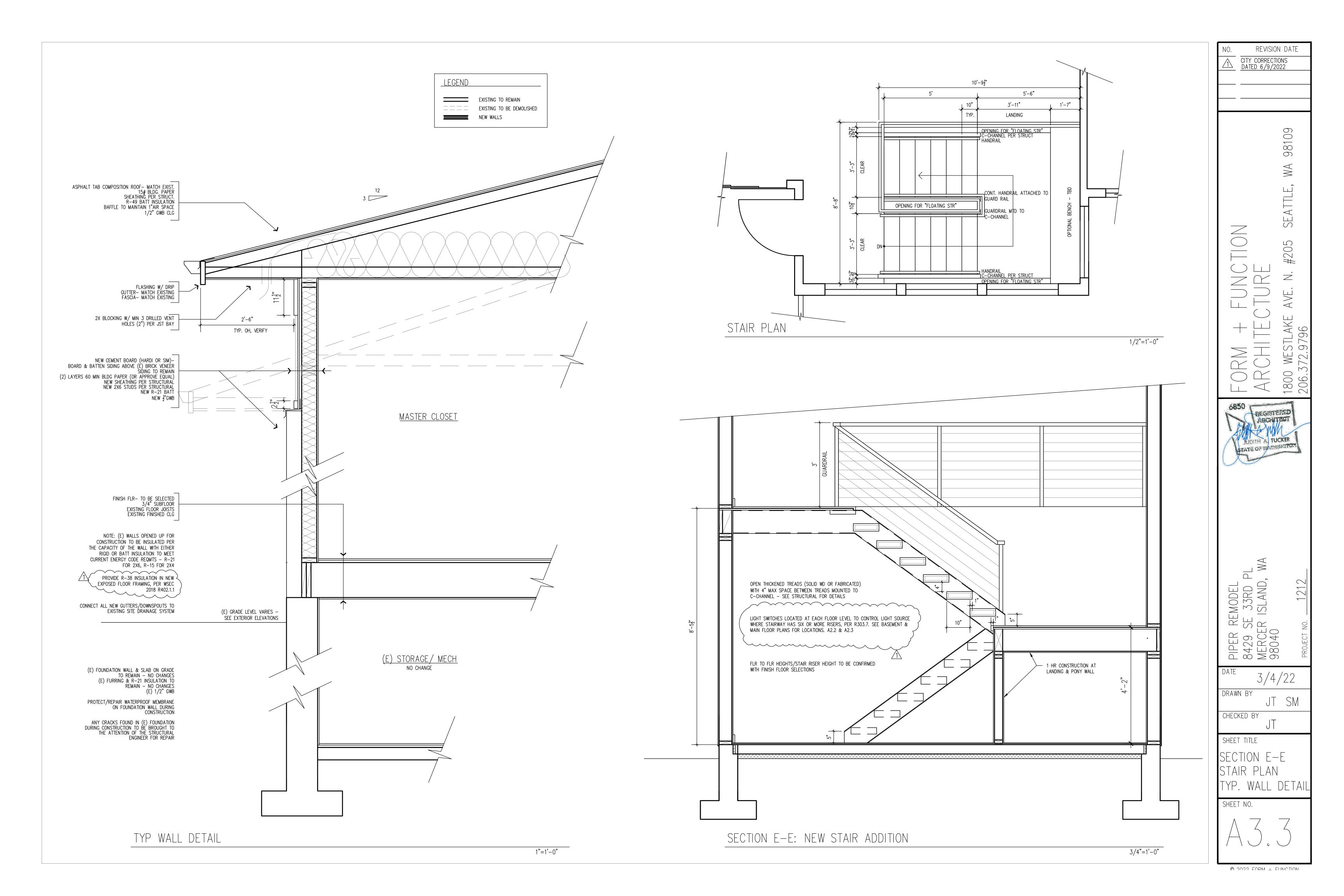
VERIFY ALL DIMENSIONS TO EXISTING ELEMENTS

98109 \triangleleft \triangleleft 05 \sim 800 $\langle \langle$ ARCHITECT JUDITH A. TUCKER STATE OF WASHINGTON REMODEL SE 33RD P ER ISLAND, PIPER 8429 MERCE 98040 DRAWN BY CHECKED BY SHEET TITLE SECTIONS DOOR SCHEDULE SHEET NO.

REVISION DATE

CITY CORRECTIONS

DATED 6/9/2022



S1.0 Structural Notes

S1.1 | Shearwall Schedule and Details

S1.2 Holddown Schedule and Details

S6.0 Typical Concrete Details

S8.0 | Moment Frame Details

S8.1 | Moment Frame Details

S8.2 | Moment Frame Details

S8.3 | Moment Frame Details

S10.0 Typical Components

S9.0 Typical Wood Framing Details

S9.1 Typical Wood Framing Details

S9.2 Typical Wood Framing Details

S2.0 Basement Level Walls Over Foundation

S2.1 Main Frmg Over Basement Lvl Shear Walls

S2.2 Roof Framing Over Main Level Shear Walls

Rev Date

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THE STRUCTURAL NOTES SUPPLEMENT THE PLANS AND SPECIFICATIONS. ANY DISCREPANCY FOUND BETWEEN THE DRAWINGS, NOTES, SPECIFICATIONS, SITE CONDITIONS, AND ARCHITECTURAL PLANS SHALL BE REPORTED TO THE ARCHITECT WHO SHALL CORRECT THE DISCREPANCY IN WRITING. ANY WORK COMPLETED AFTER DISCOVERY OF THE DISCREPANCY SHALL BE DONE AT THE CONTRACTOR'S RISK.

THE CONTRACTOR SHALL PROVIDE BRACING AND SUPPORT REQUIRED FOR TEMPORARY CONSTRUCTION LOADS AND FOR STRUCTURAL COMPONENTS AS REQUIRED DURING ERECTION. BACKFILL BEHIND WALLS SHALL NOT BE PLACED UNTIL THE WALLS ARE PROPERLY SUPPORTED.

REFER TO ARCHITECTURAL PLANS FOR OPENINGS, ARCHITECTURAL TREATMENTS, AND DIMENSIONS NOT

THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION OF ALL WORK INCLUDING BUT NOT LIMITED TO EXCAVATION, SHORING, AND OTHER WORK WITH ALL UTILITIES AND ADJACENT PROPERTIES. CALL THE UTILITY LOCATE SERVICE PRIOR TO ANY WORK AT 1-800-424-5555.

ALL DESIGN AND CONSTRUCTION SHALL CONFORM TO THE 2018 INTERNATIONAL BUILDING CODE AS ADOPTED BY SEATTLE. WASHINGTON.

Kzt = 1.3

GCpi = +/- 0.18

01100 - DESIGN LOADS

TOPOGRAPHICAL FACTOR

INTERNAL PRESSURE COEFFICIENT

WALL SYSTEM W/LIGHT FRAMED WOOD

DEAD LOADS: ACTUAL WEIGHT OF MATERIALS OF CONSTRUCTION AND PERMANENT EQUIPMENT.

SHOWN. CONSULT MECHANICAL PLANS FOR DUCTS AND PIPES ETC. NOT SHOWN.

FLOOR LIVE LOADS: 40 PSF FLOORS (RESIDENTIAL) 60 PSF **ROOF LIVE LOADS:**

SNOW LOAD DESIGN DATA: Pg = 20 PSF, Pf = 20 PSF, Ce = 0.9, Is = 1.0, Ct = 1.0, 25 PSF UNIFORM

19 20101,	11 20101,00 0.0,10	1.0, 01 1.0, 201	or ordin ordin
WIND DESIGN D	DATA:		
BASIC WIND	SPEED	100	MPH (3-SECOND GUST)
WIND IMPOR	RTANCE FACTOR	lw =	= 1.0
WIND EXPOS	SURE	EXF	POSURE B

COMPONENT/CLADDING WIND PRESSURE	P(C) = 25 PSF	=
EARTHQUAKE DESIGN DATA:		
SEISMIC IMPORTANCE FACTOR	le = 1.0	
OCCUPANCY CATEGORY	II	
SPECTRAL RESPONSE ACCELERATIONS	Ss = 1.401	S1 = 0.487

SPECTRAL RESPONSE ACCELERATIONS SITE CLASS	Ss = 1.401 D	S1 = 0.487	
SPECTRAL RESPONSE COEFFICIENTS SEISMIC DESIGN CATEGORY	SDS = 0.934 D	SD1 = 0.584	
(E-W) EQUIVELENT LATERAL FORCE - BEARING	R = 6.5	Cs = 0.14	

WALLS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR RESISTANCE T = 0.189 (LESS THAN 1.5*Ts) THEREFORE NO SITE RESPONSE ANALYSIS REQUIRED PER ASCE 7-17 SECTION 11.4.8 EXCEPTION #2.

(N-S) COMBINATION OF FRAMING SYSTEMS R = 3.5 Cs = 0.27IN THE SAME DIRECTION (ASCE 7-16 12.2.3) R = 6.5 WOOD FRAMED SHEAR WALLS, R = 3.5 ORDINARY MOMENT FRAME. GOVERNING R VALUE THIS DIRECTION = 3.5.

01200 - GEOTECNICAL INVESTIGATION

FOUNDATION DESIGN BASED ON REPORT NO. 2537.01 DATED JANUARY 28, 2022, AND SUPPLEMENTAL REPORT DATED FEBRUARY 28, 2022 PREPARED BY ZIPPER GEO., ALL SITE PREPARATION AND FOUNDATION CONSTRUCTION TO BE PERFORMED PER REPORT. ALL PILE DRIVING TO BE INSPECTED BY A CERTIFIED INSPECTOR WITH LOG CONFIRMING EACH PILE DRIVEN IN ACCORDANCE WITH SOILS REPORT REFUSAL CRITERIA. FILLS TO BE COMPACTED TO 95% MODIFIED PROCTOR PER ASTM D-1557, AND INSTALLED IN LIFTS NO GREATER THEN 10 INCHES. A MINIMUM OF 12 INCHES OF SOIL UNDER NEW INTERIOR AND EXTERIOR SLABS ON GRADE SHALL BE COMPACTED TO 95% MODIFIED PROCTOR PER ASTM D-1557.

ALL FOUNDATIONS SHALL BE FOUNDED ON PIPE PILE OR BY ON AT LEAST MEDIUM DENSE / STIFF NATIVE SOILS OR ABOVE PROPERLY COMPACTED STRUCTURAL FILL OR CDF WITH 100 PSI COMPRESSIVE STRENGTH PLACED ABOVE ADEQUATE NATIVE SOILS PER THE DISCRESSION OF THE GEOTECHNICAL ENGINEER. WHERE FOUNDATIONS ARE FOUNDED ATOP CONDITIONS DESCRIBED ABOVE, AN ALLOWABLE NET BEARING CAPACITY OF 2000 PSF HAS BEEN USED FOR DESIGN.

GEOTECHNICAL DESIGN PARAMETERS HAVE BEEN COORDINATED WITH ZIPPER GEO AS LISTED BELOW

DESIGN PARAMETERS ARE AS FOLLOWS: 400 PCF (ULTIMATE) PASSIVE EARTH PRESSURE COEFFICIENT OF FRICTION 0.5 (ULTIMATE)

ALL FOUNDATION INSTALLLATIONS SHALL BE SUBJECT TO APPROVAL OF THE GEOTECHNICAL ENGINEER.

SITE CLASS D

INSTALLATION REQUIREMENTS:

SOIL PROFILE

TWO AND THREE INCH DIAMETER PIPE PILE SHALL CONSIST OF PIPE PER ASTM A53 GRADE B AND BE DRIVEN AT LEAST 10 FEET INTO COMPETENT SOIL. PIPE PILE REACHING THE FOLLOWING PENETRATION RATES MAY BE ASSIGNED THE FOLLOWING COMPRESSIVE CAPACITIES. PIPE PILE SHALL BE INSTALLED USING A HYDRAULIC IMPACT HAMMER CARRIED ON LOADS THAT ALLOW THE HAMMER TO SIT ON THE TOP OF THE PILE DURING DRIVING. IF ALTERNATE DRIVING MEATHODS ARE USED, COORDINATE REQUIRED LOAD TESTS WITH GEOTECHNICAL ENGINEER. GEOTECHNICAL SPECAIL INSPECTOR SHALL BE CONTINOUSLY PRESENT DURING PIPE PILE INSTALLATION. (F.D.R. - FINAL DRIVING RATE):

PILE DIAMETER	F.D.R. 90 LB. PERCUSSION DRIVER	F.D.R. 650 LB PERCUSSION DRIVER	ALLOWABLE COMP. CAPACITY
2 INCH (X-STRONG) SCHEDULE 80	60 SEC/INCH	NA	3 TONS (6,000 LB)
3 INCH (STANDARD) SCHEDULE 40	NA	15 SEC/INCH	6 TONS (12,000 LB)
(E) 4 INCH	NA	NA	10 TONS (20 000 LB)

FIELD TESTING REQUIREMENTS:

LOAD TESTS ARE NOT REQUIRED FOR TWO OR THREE INCH DIAMETER PIPE PILES THAT ARE DRIVEN IN ACCORDANCE WITH THE RECOMMENDATIONS PRESENTED IN JANUARY 28, 2022 REPORT PREPARED BY ZIPPER GEO, AND PROVIDED THAT A ZGA REPRESENTATIVE OBSERVES INSTALLATION OF THE PILES AND VERIFIES THAT REFUSAL HAS BEEN ACHIEVED.

01300 - SHOP DRAWING SUBMITTAL PROCESS SHOP DRAWINGS ARE TO BE SUBMITTED TO THE ARCHITECT AND ENGINEER OF RECORD FOR APPROVAL PRIOR TO FABRICATION. IF SHOP DRAWINGS DIFFER FROM THE APPROVED DESIGN DRAWINGS, NEW DESIGN DRAWINGS BEARING THE SEAL AND SIGNATURE OF A LICENSED STATE OF WASHINGTON STRUCTURAL ENGINEER SHALL BE SUBMITTED ALONG WITH THE SHOP DRAWINGS TO THE BUILDING OFFICIAL FOR APPROVAL PRIOR TO FABRICATION.

SHOP DRAWINGS ARE REQUIRED FOR STRUCTURAL STEEL AND PROPRIETARY GUARD COMPONENT.

01400 - INSPECTIONS AND SPECIAL INSPECTIONS THE CONTRACTOR SHALL BE RESPONSIBLE TO COORDINATE ALL INSPECTIONS REQUIRED BY THE LOCAL BUILDING DEPARTMENT.

SPECIAL INSPECTIONS ARE NOT REQUIRED FOR GROUP R-3 OCCUPANCIES UNLESS OTHERWISE REQUIRED BY THE BUILDING OFFICIAL.

01600 - QUALITY ASSURANCE REQUIREMENTS THE QUALITY ASSURANCE PLAN SHALL BE TO VERIFY THAT THE SPECIAL INSPECTIONS NOTED IN SECTION 01400 AND THE STRUCTURAL OBSERVATION NOTED IN SECTION 01500 HAVE BEEN COMPLETED AND THAT SUPPORTING DOCUMENTATION NOTED IN SUCH SECTIONS HAS BEEN PROVIDED.

SPECTRAL RESPONSE AT SHORT PERIODS, SDS, NOT EXCEEDING 0.50g. QUALITY ASSURANCE PLAN IS NOT REQUIRED FOR WIND EXPOSURE B WHERE BASIC WIND SPEED IS LESS

QUALITY ASSURANCE PLAN IS NOT REQUIRED FOR STRUCTURES OF LIGHT WOOD FRAMING WITH DESIGN

THAN 120 MPH. SUMMARY: A QUALITY ASSURANCE PLAN IS NOT REQUIRED BY CODE FOR THIS STRUCTURE. 01700 - EXECUTION REQUIREMENTS INSTALLATION OF ALL STRUCTURAL COMPONENTS SHALL BE AS REQUIRED PER ALL LOCAL CODES.

02000: SITE CONSTRUCTION

ALL SITE CONSTRUCTION SHALL BE CONSISTENT WITH THE GEOTECHNICAL ENGINEERING RECOMMENDATIONS AS NOTED IN THE GEOTECHNICAL ENGINEERING REPORT (SEE SECTION 01200) AND IN SUBSEQUENT DIRECTIVES.

02100 - EXCAVATION SUPPORT AND PROTECTION

EXCAVATION FOR FOUNDATIONS SHALL BE PER PLAN DOWN TO UNDISTURBED NATIVE MATERIAL PER THE GEOTECHNICAL ENGINEERING RECOMMENDATIONS. OVER-EXCAVATED AREAS SHALL BE BACKFILLED WITH LEAN CONCRETE OR PER GEOTECHNICAL RECOMMENDATIONS AT THE CONTRACTOR'S EXPENSE.

EXCAVATION SLOPES SHALL BE SAFE AND SHALL NOT BE GREATER THAN THE LIMITS SPECIFIED BY LOCAL, STATE, AND NATIONAL SAFETY REGULATIONS.

INSTALLATION OF CONSTRUCTION SHORING, IF REQUIRED, SHALL BE PER THE SHORING DRAWINGS, NOTES, AND SPECIFICATIONS

02200 - BACKFILL AND COMPACTION

BACKFILL SHALL NOT BE PLACED UNTIL THE REMOVAL OF FORMWORK AND OF ANY DEBRIS. BACKFILL BEHIND ALL WALLS SHALL NOT BE PLACED UNTIL THE WALLS ARE PROPERLY SUPPORTED. ALL BACKFILL MATERIAL AND PLACEMENT PROCEDURES SHALL BE CONSISTENT WITH THE GEOTECHNICAL ENGINEERING RECOMMENDATIONS.

03000 - CAST-IN-PLACE CONCRETE CONCRETE CONSTRUCTION SHALL CONFORM TO THE AMERICAN CONCRETE INSTITUTE STANDARD ACI 318-14 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE".

CEMENT AND CONCRETE SHALL CONFORM TO IBC SECTION 1903. ADMIXTURES SHALL BE APPROVED BY THE ENGINEER OF RECORD AND SHALL COMPLY WITH ACI 318-14 SECTION 3.6. CONCRETE EXPOSED TO FREEZING AND THAWING SHALL HAVE AN AIR ENTRAINING ADMIXTURE CONFORMING TO IBC SECTION 1904.2. THE USE OF WATER SOLUBLE CHLORIDE ION SHALL

CONCRETE MIX DESIGNS SHALL MEET THE FOLLOWING REQUIREMENTS (1) 28 DAY MAX. STRENGTH fc [PSI] (2) MAX. WATER / CEMENT RATIO (3) MAX. SLUMP [IN] (4) AIR ENTRAINMENT [%] (5) SPECIAL INSPECTION REQUIRED (6) MIN. 90 LB SACKS OF CEMENT (7) LOCATION AND

(1)	(2)	(3)	(4)	(5)	(6)	(7)
3000	0.45	4+/-1	5+/-1	NO		EXTERIOR SLAB ON GRADE
3000	0.41	4+/-1	0+/-1	NO		INTERIOR SLAB ON GRADE
3000	0.50	5+/-1	0+/-1	NO		FOOTINGS AND GRADE BEAMS
3000	0.45	5+/-1	5+/-1	NO		STEMS
3000	0.50	5+/-1	5+/-1	NO		ALL OTHER CONCRETE

SPECIAL INSPECTION IS NOT REQUIRED AS THE DESIGN IS BASED ON fc = 2500 PSI.

CHAMFER ALL EXPOSED CORNERS PER THE ARCHITECTURAL PLANS OR 3/4 INCH IF NOT SPECIFIED BY THE ARCHITECT.

03100 - REINFORCING STEEL

APPLICATION.

REINFORCING STEEL DETAILING, FABRICATION, AND PLACEMENT SHALL BE PER ACI 318-14. REINFORCING STEEL SHALL MEET THE FOLLOWING REQUIREMENTS:

ASTM A-615 DEFORMED BARS GRADE 40 (fy=40 KSI) FOR #3 BARS ONLY ASTM A-615 DEFORMED BARS GRADE 60 (fy=60 KSI) FOR #4 BARS AND LARGER ASTM A-706 DEFORMED BARS GRADE 60 (fy=60 KSI) FOR ALL WELDABLE BARS ASTM A-1064 SMOOTH BAR (fy=60 KSI) FOR WELDED WIRE FABRIC

REINFORCING FOR SLABS ON GRADE SHALL BE 6X6 W1.4XW1.4 WELDED WIRE FABRIC OR FIBER MESH UNLESS NOTED OTHERWISE. PROVIDE LAP SPLICES PER THE LAP SPLICE SCHEDULE ON SHEET S6.0. REINFORCING STEEL AT ALL WALLS, SLABS, AND FOOTINGS SHALL BE CONTINUOUS AROUND CORNERS ELSE CORNER BARS SHALL BE PROVIDED.

COVER REQUIREMENTS SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE:

CONCRETE CAST AGAINST EARTH ALL BAR SIZES . . FORMED SURFACE EXPOSED TO EARTH OR WEATHER #6 AND LARGER . . #5 AND SMALLER CONCRETE NOT EXPOSED TO EARTH OR WEATHER WALLS AND JOISTS #14 AND #18 BARS . . .1 1/2" #11 BARS AND SMALLER 3/4"

SLABS AND JOISTS #14 AND #18 BARS. #11 BARS AND SMALLER . . BEAMS, COLUMNS PRIMARY REINFORCEMENT 1 1/2"

TIES, STIRRUPS, AND SPIRALS ... 1 1/2" REINFORCING STEEL SHALL BE ACCURATELY PLACED AND ADEQUATELY SECURED IN PLACE PRIOR TO CONCRETE PLACEMENT. REINFORCING STEEL SHALL NOT BE FIELD BENT EXCEPT AS NOTED IN THE DESIGN DRAWINGS. WELDING OF REINFORCING STEEL SHALL NOT BE PERMITTED WITHOUT PRIOR

APPROVAL OF THE ENGINEER OF RECORD EXCEPT AS NOTED ON THE DESIGN DRAWINGS.

03200 - CONCRETE WALL REINFORCING PLACE TWO HORIZONTAL #5 BARS AT EACH FLOOR LEVEL OR TOP OF WALL ELEVATION. PROVIDE CORNER BARS TO MATCH HORIZONTAL REINFORCEMENT AT EACH WALL CORNER AND INTERSECTION. PROVIDE TWO VERTICAL #5 BARS AT EACH WALL CORNER AND INTERSECTION. AT ALL WALL OPENINGS PROVIDE TWO #5 BARS OVER, UNDER, AND AT THE SIDES OF THE OPENINGS, EXTEND THE HORIZONTAL BARS THE LAP SPLICE DISTANCE PAST THE OPENING OR EXTEND AS FAR AS POSSIBLE AND HOOK. PROVIDE ONE #5 BAR BY 4'-0" LONG DIAGONALLY AT EACH CORNER OF THE WALL OPENING. ALL CONCRETE SHALL BE PLACED AND CONSOLIDATED WALLS SHALL BE REINFORCED PER SCHEDULE

BELOW U.N.O.:			
VALL THICKNESS	HORIZONTAL #4 AT 14"OC #4 AT 10"OC	#5 AT 18"OC #5 AT 15"OC	LOCATION CENTERLINE CENTERLINE
0"	#4 AT 16"OC	#5 AT 18"OC	EACH FACE

#4 AT 12"OC #5 AT 18"OC EACH FACE EPOXY ALL HORIZONTAL STEEL INTO EXISTING FOUNDATION WITH FOUR INCH EMBEDMENT. RE: NOTES

SECTION 08100 FOR EPOXY TYPE.

DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION "AISC 360-10 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS". MATERIALS SHALL BE IN ACCORDANCE WITH THE FOLLOWING

U.N.O.:		
STRUCTURAL W SHAPE	ASTM A-992	Fy = 50 KSI
S, M, AND C SHAPES	ASTM A-36	Fy = 36 KSI
STEEL ANGLES	ASTM A-36	Fy = 36 KSI
PLATE MATERIAL	ASTM A-36	Fy = 36 KSI
STRUCTURAL PIPE	ASTM A-53 GRADE B	Fy = 35 KSI
STRUCTURAL HSS	ASTM A-500 GRADE B	Fy = 46 KSI
ANCHOR RODS	ASTM F1554	Fy = 36 KSI
WOOD CONNECTION BOLTS	ASTM A-307 GRADE A	-

WELDING ELECTRODES E7018 ALL WELDING SHALL CONFORM TO THE AWS D1.4 "STRUCTURAL WELDING CODE". ALL WELDING SHALL BE PERFORMED BY A WASHINGTON ASSOCIATION OF BUILDING OFFICIALS (WABO) AND AMERICAN WELDING SOCIETY (AWS) CERTIFIED WELDERS. ALL COMPLETE PENETRATION (CP) WELDS SHALL BE ULTRASONICALLY TESTED. ALL FILLET WELDS SHALL BE VISUALLY INSPECTED RE: S1.1.

STRUCTURAL STEEL AND CONNECTIONS EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED AFTER FABRICATION IN COMPLIANCE WITH ASTM A-123. ALL FIELD WELDS EXPOSED TO WEATHER SHALL BE COATED WITH BRUSH APPLIED ZINC-RICH PAINT COMPLYING WITH ASTM A-780.

ALL STRUCTURAL STEEL TO RECEIVE ONE COAT OF PAINT (PRIME COAT). PROVIDE A MINIMUM FRY-FILM THICKNESS OF ONE MIL. PREPARE SURFACE TO MEET REQUIREMENTS OF SSPC-SP2. TOUCHUPS OF ABRASIONS ARE THE RESPONSIBILITY OF THE CONTRACTOR. UNO. REFER TO ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR ADDITIONAL INFORMATION RELATING TO FINISH PAINT OR OTHER FINISH REQUIREMENTS.

06000 - WOOD FRAMING NOTES FRAMING CONNECTORS, ACCESSORIES, AND FASTENERS AS NOTED IN THE PLANS AND DETAILS SHALL BE AS MANUFACTURED BY SIMPSON STRONG-TIE. EQUIVALENT HARDWARE MY BE USED WITH PRIOR APPROVED BY ENGINEER OF RECORD. INSTALL ALL HARDWARE PER MANUFACTURERS SPECIFICATIONS. WHERE STRAPS CONNECT TWO MEMBERS TOGETHER. PLACE HALF OF THE REQUIRED FASTENERS INTO

EACH MEMBER. PROVIDE SOILD BLOCKING AT ALL BEARING POINTS. SEE SECTION 06100 FOR FASTENER REQUIREMENTS AT TREATED LUMBER. TYPICAL NAILING NOT SHOWN PER PLAN, DETAIL, OR SCHEDULE SHALL CONFORM TO FASTENING SCHEDULE PER IBC TABLE 2304.10.1 OR TO THE FASTENING SCHEDULE ON

NAILS SHALL BE COMMON UNLESS NOTED OTHERWISE COMMON NAIL DIMENSIONS ARE AS FOLLOWS:

NAIL SIZE	DIAMETER	LENGTH
8d	0.131"	2 1/2"
10d	0.148"	3"
12d	0.148"	3 1/4"
16d	0.162"	3 1/2"

UNLESS NOTED OTHERWISE PER SHEARWALL SCHEDULE OR PLANS. ANCHOR BOLTS AT SILL PLATES SHALL BE 5/8 INCH DIAMETER WITH 7 INCHES MINIMUM EMBEDMENT INTO CONCRETE AND SHALL BE SPACED NOT MORE THAN 4 FEET APART. THERE SHALL BE A MINIMUM OF TWO BOLTS PER SILL PIECE WITH ONE BOLT LOCATED NOT MORE THAN 12 INCHES NOR LESS THAN 4 1/2 INCHES FROM EACH END OF THE PIECE. A 3"x3"x1/4" PLATE WASHER SHALL BE PROVIDED FOR ALL ANCHOR BOLTS (COUNTERSINK PLATE WASHERS SHALL NOT BE ALLOWED).

06100 - ROUGH FRAMING SAWN LUMBER SHALL CONFORM TO WEST COAST LUMBER INSPECTION BUREAU (WCLIB) "GRADING AND DRESSING RULES" NO. 17 LATEST EDITION. SAWN LUMBER SHALL BE S4S AND SURFACED DRIED. 19 PERCENT MAXIMUM MOISTURE CONTENT. PROTECT LUMBER FROM WEATHER AND PROVIDE FURTHER DRYING OF ASSEMBLED FRAMING TO MINIMIZE WOOD SHRINKAGE POTENTIAL. ALL LUMBER EXPOSED TO WEATHER OR IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESERVATIVE TREATED U.N.O. PER PLAN. LUMBER SPECIES, GRADE, AND PROPERTIES FOR EACH USE/LOCATION SHALL BE AS FOLLOWS U.N.O. PER PLAN/SCHEDULE:

USE/LOCATION	SPECIES	GRADE	Fb (PSI)	Fv (PSI)	Fcp (PSI)	Fc (PSI)	E (PSI)
2X, 3X 4" WIDE	HEM-FIR	STUD	675	150	405	800	1.2E6
2X, 3X 6" & WIDER	HEM-FIR	NO. 2	850	150	405	1300	1.3E6
WALL PLATES 2X4, 3X4 2X6, 3X6	HEM-FIR HEM-FIR	STUD NO. 2	675 850	150 150	405 405	800 1300	1.2E6 1.3E6
JOISTS 2X, 3X	HEM-FIR	NO. 2	850	150	405	1300	1.3E6
LEDGERS 2X, 3X 4X	DOUGLAS FIR-LARCH DOUGLAS FIR-LARCH	NO. 2 NO. 1					1.6E6 1.7E6
BEAMS AND POSTS 4X 6X	DOUGLAS FIR-LARCH DOUGLAS FIR-LARCH	NO. 2 NO. 1					1.6E6 1.6E6
	WALL STUDS/BLOCKIN 2X, 3X 4" WIDE 2X, 3X 6" & WIDER WALL PLATES 2X4, 3X4 2X6, 3X6 JOISTS 2X, 3X LEDGERS 2X, 3X 4X BEAMS AND POSTS 4X	WALL STUDS/BLOCKING 2X, 3X HEM-FIR 4" WIDE 2X, 3X HEM-FIR 6" & WIDER WALL PLATES 2X4, 3X4 HEM-FIR 2X6, 3X6 HEM-FIR JOISTS 2X, 3X HEM-FIR LEDGERS 2X, 3X DOUGLAS FIR-LARCH 4X DOUGLAS FIR-LARCH BEAMS AND POSTS 4X DOUGLAS FIR-LARCH	WALL STUDS/BLOCKING 2X, 3X HEM-FIR STUD 4" WIDE 2X, 3X HEM-FIR NO. 2 6" & WIDER WALL PLATES 2X4, 3X4 HEM-FIR STUD 2X6, 3X6 HEM-FIR NO. 2 JOISTS 2X, 3X HEM-FIR NO. 2 LEDGERS 2X, 3X DOUGLAS FIR-LARCH NO. 1 BEAMS AND POSTS 4X DOUGLAS FIR-LARCH NO. 2	USE/LOCATION SPECIES GRADE (PSI) WALL STUDS/BLOCKING 2X, 3X HEM-FIR STUD 675 4" WIDE 2X, 3X HEM-FIR NO. 2 850 6" & WIDER WALL PLATES 2X4, 3X4 HEM-FIR STUD 675 2X6, 3X6 HEM-FIR NO. 2 850 JOISTS 2X, 3X HEM-FIR NO. 2 850 LEDGERS 2X, 3X DOUGLAS FIR-LARCH NO. 2 900 900 4X DOUGLAS FIR-LARCH NO. 1 1000 NO. 1 1000	USE/LOCATION SPECIES GRADE (PSI) (PSI) WALL STUDS/BLOCKING 2X, 3X HEM-FIR STUD 675 150 4" WIDE 2X, 3X HEM-FIR NO. 2 850 150 6" & WIDER WALL PLATES STUD 675 150 2X4, 3X4 HEM-FIR STUD 675 150 2X6, 3X6 HEM-FIR NO. 2 850 150 JOISTS 2X, 3X HEM-FIR NO. 2 850 150 LEDGERS 2X, 3X DOUGLAS FIR-LARCH NO. 2 900 180 180 4X DOUGLAS FIR-LARCH NO. 1 1000 180 180 BEAMS AND POSTS 4X DOUGLAS FIR-LARCH NO. 2 900 180	USE/LOCATION SPECIES GRADE (PSI) (PSI) WALL STUDS/BLOCKING 2X, 3X HEM-FIR STUD 675 150 405 4" WIDE 2X, 3X HEM-FIR NO. 2 850 150 405 EX, 3X HEM-FIR STUD 675 150 405 WALL PLATES 2X4, 3X4 HEM-FIR STUD 675 150 405 2X6, 3X6 HEM-FIR NO. 2 850 150 405 JOISTS 2X, 3X HEM-FIR NO. 2 850 150 405 LEDGERS 2X, 3X DOUGLAS FIR-LARCH NO. 2 900 180 625 BEAMS AND POSTS 4X DOUGLAS FIR-LARCH NO. 2 900 180 625	USE/LOCATION SPECIES GRADE (PSI) (PSI) (PSI) (PSI) WALL STUDS/BLOCKING 2X, 3X HEM-FIR STUD 675 150 405 800 4" WIDE 2X, 3X HEM-FIR NO. 2 850 150 405 1300 6" & WIDER WALL PLATES 2X4, 3X4 HEM-FIR STUD 675 150 405 800 2X6, 3X6 HEM-FIR NO. 2 850 150 405 1300 JOISTS 2X, 3X HEM-FIR NO. 2 850 150 405 1300 LEDGERS 2X, 3X HEM-FIR NO. 2 850 150 405 1300 LEDGERS 2X, 3X DOUGLAS FIR-LARCH NO. 2 900 180 625 1350 4X DOUGLAS FIR-LARCH NO. 1 1000 180 625 1500 BEAMS AND POSTS 4X DOUGLAS FIR-LARCH NO. 2 900 180 625 1350

06200 - PRESERVATIVE TREATED WOOD PRODUCTS

PRESERVATIVE TREATED WOOD SHALL BE REQUIRED FOR ALL WOOD THAT FORMS THE STRUCTURAL SUPPORT OF THE BUILDING, BALCONIES PORCHES, OR SIMILAR PERMANENT BUILDING APPURTENANCES THAT ARE EXPOSED TO THE WEATHER WITHOUT ADEQUATE PROTECTION FROM A ROOF, EAVE, OVERHANG OR OTHER COVERING TO PREVENT MOISTURE OR WATER ACCUMULATION AT THE SURFACE OR AT JOINTS BETWEEN MEMBERS.

ALL WOOD INSTALLED ABOVE GROUND AND RESTING ON AN EXTERIOR CONCRETE OR MASONRY FOUNDATION WALL LESS THAN 8 INCHES FROM EXPOSED EARTH.

POSTS OR COLUMNS SUPPORTING PERMANENT STRUCTURES AND SUPPORTED BY A CONCRETE SLAB OR FOOTING THAT IS IN DIRECT CONTACT WITH THE EARTH. EXCEPT:

1. IF LOCATED IN BASEMENTS ON A CONCRETE PIER OR METAL PEDESTAL 1 INCH ABOVE THE SLAB AND SEPARATED THEREFROM BY AN IMPERVIOUS MOISTURE BARRIER. IF IN AN ENCLOSED CRAWL SPACE OR AN UNEXCAVATED AREA WITHIN THE BUILDING

PERIPHERY AND SUPPORTED BY A CONCRETE PIER OR PEDESTAL MORE THAN 8 INCHES FROM EXPOSED GROUND AND SEPARATED THEREFROM BY AN IMPERVIOUS MOISTURE BARRIER. SLEEPERS AND SILLS ON A CONCRETE SLAB ON GRADE THAT DOES NOT HAVE AN IMPERVIOUS

MOISTURE BARRIER SEPARATION WITH EXPOSED EARTH. LEDGERS AND FURRING ATTACHED DIRECTLY TO THE INTERIOR OF EXTERIOR CONCRETE OR

MASONRY WALLS BELOW GRADE. PRESERVATIVE TREATMENT SHALL BE PER AMERICAN WOOD PRESERVERS' ASSOCIATION (AWPA) SPECIFICATION C2 AND C9 OR APPLICABLE STANDARDS.

ALL FASTENERS (NAILS, BOLTS, ANCHOR BOLTS, PLATES, HANGERS, ETC.) IN CONTACT

WITH TREATED LUMBER SHALL BE CORROSION RESISTANT G-185 HOT DIPPED GALVANIZED PER ASTM

UNLESS NOTED OTHERWISE PER PLANS OR DETAILS:

MEMBER SIZE

JOIST AND BEAM HANGERS AS NOTED IN THE PLANS SHALL BE AS MANUFACTURED BY SIMPSON STRONG-TIE. EQUIVALENT HARDWARE MAY BE USED WITH PRIOR APPROVAL BY ENGINEER OF RECORD. JOIST AND BEAM HANGERS SHALL BE INSTALLED PER MANUFACTURERS' SPECIFICATIONS AND SHALL BE AS FOLLOWS

			10.00-10		
SAWN LUMBER			LUS OR HUS SERIE	S TO MATCH LUMB	BER SIZE
			WHERE NOT NOTE	D SPECIFICALLY BE	ELOW
	GLUED	LAMINATED BEAMS (H = BEAM	M DEPTH TYPICAL) (D	OF CAPACITY / HF C	CAPACITY)
	3 1/8"	LGU3.25-SDS W/(16) SDS 1/4x2	2 1/2" FACE, (12) SĎŠ	1/4x2 1/2" JOIST	(6720 / 4840)
	3 1/2"	HGU3.63-SDS W/(36) SDS 1/4x2	2 1/2" FACE, (24) SDS	1/4x2 1/2" JOIST	(14145 / 1018
	5 1/8"	HGU5.25-SDS W/(36) SDS 1/4x2	2 1/2" FACE, (24) SDS	1/4x2 1/2" JOIST	(14145 / 1018
	5 1/4"	HHGU5.50-SDS W/(44) SDS 1/4	x2 1/2" FACE, (28) SD	S 1/4x2 1/2" JOIST	(17845 / 1285
	5 1/2"	HHGU5.62-SDS W/(44) SDS 1/4	x2 1/2" FACE. (28) SD	S 1/4x2 1/2" JOIST	(17845 / 1285

HANGER

8 3/4" HHGU9.00-SDS W/(44) SDS 1/4x2 1/2" FACE, (28) SDS 1/4x2 1/2" JOIST (17845 / 12850) 10 3/4" HHGU11.00-SDS W/(44) SDS 1/4x2 1/2" FACE, (28) SDS 1/4x2 1/2" JOIST (18480 / 13305) 1 1/2" x 11 7/8" MIU1.56/11 W/(20) 16d FACE, (2) 10d x 1 1/2" JOIST (2880) (2) 1 3/4" x 11 7/8" HHUS410 W/(30) 16d FACE, (10) 16d JOIST

6 3/4" HHGU7.00-SDS W/(44) SDS 1/4x2 1/2" FACE, (28) SDS 1/4x2 1/2" JOIST (17845 / 12850)

3 1/2" x 11 7/8" (5635)HHUS410 W/(30) 16d JOIST PROVIDE HUC HANGER FOR BEAM SIZE SPECIFIED FOR END OF BEAM CONDITIONS.

06400 - SHRINKAGE OF WOOD FRAMING SHRINKAGE IN WOOD FRAMING IS DUE TO LOSS OF MOISTURE CONTENT AND TO COMPRESSION OF ASSEMBLIES OF WOOD COMPONENTS. PLUMBING, ELECTRICAL, AND MECHANICAL SYSTEMS AS WELL AS EXTERIOR FINISHES SHALL BE DESIGNED AND BUILT TO ACCOMMODATE 1/4 INCH PER FLOOR WOOD SHRINKAGE. THE USE OF KILN DRIED LUMBER AND PROVIDING A DRYING PROCESS TO THE FRAMING MEMBERS PRIOR TO APPLICATION OF FINISHES WILL HELP CONTROL BUT WILL NOT ELIMINATE SHRINKAGE.

06500 - WOOD SHEATHING

STRUCTURAL WOOD SHEATHING PANELS SHALL HAVE APA GRADE TRADEMARK OF THE AMERICAN PLYWOOD ASSOCIATION. WOOD SHEATHING PANELS SHALL BE C-D INT APA WITH EXTERIOR GLUE (CDX). ORIENTED STRAND BOARD (OSB) PANELS SHALL BE EXPOSURE 1. PANELS SHALL HAVE THE FOLLOWING THICKNESS, SPAN RATING, AND FASTENING UNLESS NOTED OTHERWISE PER PLAN:

11011111200, 01 71	, , , , , , , , , , , , , , , , , , ,	/ IND I / IO I EI III IO	CINELOC	NOTEDOTI	ILITATION I
				EDGE	FIELD
				NAILS	NAILS
00F:	5/8" 40/20	C-D APA CDX		8d AT 6"	8d AT 12"
_OOR:	3/4" 48/24	C-D T&G		10d AT 6"	10d AT 12"
HEARWALL:	7/16" C-D	EXTERIOR GLUE		SEE SCHEE	DULE SHEET S
XTERIOR WALL:	7/16" D-D	EXTERIOR GLUE		10d AT 6"	10d AT 12"

ALL ROOF SHEATHING PANELS SHALL BE INSTALLED FACE GRAIN PERPENDICULAR TO SUPPORTS AND IN A STAGGERED PATTERN UNLESS NOTED OTHERWISE PER PLAN. BLOCKING AT INTERMEDIATE FLOOR AND ROOF SHEATHING JOINTS SHALL NOT BE REQUIRED UNLESS NOTED OTHERWISE PER PLAN. SHEARWALL SHEATHING SHALL BE BLOCKED AT ALL EDGES WITH 2X OR 3X FRAMING PER SHEARWALL SCHEDULE.

MICROLAM LAMINATED VENEER LUMBER (LVL) PARALLAM PARALLEL STRAND LUMBER (PSL) 2400 NA NA COLUMN BEAM 2900 290 750

06610 - SHOP FABRICATED METAL PLATE CONNECTED WOOD TRUSSES

DEFLECTION CRITERIA SHALL BE AS FOLLOWS:

TOP CHORD LOADS

TOP CHORD LIVE LOAD

TOP CHORD DEAD LOAD

CORNERS

EDGES

ABOVE PRESSURES LESS

BOTTOM CHORD LOADS

DEFLECTION LIMITATIONS

TOTAL LOAD DEFLECTION

LIVE LOAD DEFLECTION

SUPPORT OF TRUSS.

SIMPLE SPAN BEAM

CONTINUOUS BEAM

CANTILEVER BEAM

OF RECORD APPROVAL.

FOLLOWS:

06620 - STRUCTURAL GLUED LAMINATED TIMBER

06630 - STRUCTURAL COMPOSITE LUMBER (SCL)

ORIENTATION TIMBERSTRAND LAMINATED STRAND LUMBER (LSL)

COLUMN

BOTTOM CHORD DEAD LOAD

FIELD

TOP CHORD GROSS WIND UPLIFT

OVERHANG AT EDGE

TOP CHORD NET WIND PRESSURE

OVERHANGS AT CORNERS

PREMANUFACTURED METAL-PLATE-CONNECTED WOOD TRUSSES SHALL BE DESIGNED AND

ANSI/TPI 1-2007 "NATIONAL DESIGN STANDARD FOR METAL-PLATE-CONNECTED WOOD TRUSS

STAMP AND SEAL OF A REGISTERED STATE OF WASHINGTON PROFESSIONAL ENGINEER.

CONSTRUCTION". A TRUSS SUBMITTAL PACKAGE SHALL BE SUBMITTED FOR APPROVAL PRIOR TO

MANUFACTURED IN ACCORDANCE WITH IBC SECTION 2303.4 TRUSSES, AND THE TRUSS PLATE INSTITUTE

FABRICATION PER THE REQUIREMENTS OF IBC 2303.4.2. THE TRUSS DESIGN DRAWINGS SHALL BEAR THE

DESIGN FOR THE SPANS, LOADS, SHAPES, BEARING POINTS, INTERSECTIONS, HIPS AND VALLEYS, OVER-

FRAMING, BLOCKING PANELS AND ALL CONDITIONS SHOWN ON THE PLANS. THE DESIGN LOADS AND

25 PSF

10 PSF

44.8 PSF

32.0 PSF

44.8 PSF

32.0 PSF

22.3 PSF

10.0 PSF

5 PSF

PROVIDE ALL TRUSS-TO-TRUSS CONNECTION DETAILS INCLUDING BLOCKING PANELS AND REQUIRED

MATERIALS. PROVIDE EACH TRUSS WITH THE STRUCTURAL BUILDING COMPONENT (SBCA) TAGS FOR

BEARING LOCATIONS, PERMANENT BRACING LOCATIONS ETC.. THE TRUSS DESIGNER SHALL SPECIFY ALL

STORE. INSTALL & BRACE TRUSSES IN ACCORDANCE WITH WTCA/TPI (SBCA) BUILDING COMPONENT SAFETY INFORMATION (BCSI) "GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING & BRACING OF METAL-

PLATED-WOOD TRUSSES" & BCSI B1 THROUGH B11 QUICK REFERENCES. THE CONTRACTOR SHALL INSTALL

L/720

PERMANENT BRACING LOCATIONS & TRUSS REACTIONS ON THE TRUSS DESIGN DRAWINGS.

ALL TEMPORARY BRACING; SEE BCSI-2 FOR TYPICAL TEMPORARY BRACING REQUIREMENTS.

THE CONTRACTOR SHALL INSTALL ALL PERMANENT BRACING AS INDICATED ON THE TRUSS DESIGN

MINIMUM BEARING FOR TRUSSES SHALL BE 3 1/2". SECURE TRUSSES TO TOP PLATE WITH (2) 0.148" DIAMETER x 3" TOE NAILED, ONE EACH SIDE. AS A MINIMUM PROVIDE H2.5A HURRICANE CLIP AT EACH

GLUED-LAMINATED MEMBERS SHALL HAVE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION (AITC)

IMMEDIATELY AFTER TRIMMING IN EITHER SHOP OR FIELD. DESIGN MATERIAL PROPERTIES SHALL BE AS

SPECIES

DF/DF

DF/DF

DF/DF

1700 400 680

2325 310 800

2325 310 800 2050 1.55E6

CAMBER

STANDARD

2050 1.55E6

2500 1.9E6

2500 1.8E6

2900 2.0E6

IDENTIFICATION MARK. EXPOSED MEMBERS SHALL RECEIVE ONE COAT OF END SEALER APPLIED

UNEXPOSED GLUED-LAMINATED TIMBER SHALL BE INDUSTRIAL GRADE. TYPICAL, UNLESS NOTED

APPROVED EQUAL. STORAGE, ERECTION, AND INSTALLATION SHALL BE PER MANUFACTURER

ALLAOWABLE DESIGN MATERIAL PROPERTIES SHALL BE AS FOLLOWS (ALL UNITS ARE IN PSI):

OTHERWISE. EXPOSED GLUED LAMINATED TIMBER SHALL BE APPEARANCE CLASS PER ARCHITECT

STRUCTURAL COMPOSITE LUMBER SHALL CONFORM TO ALL PERTINENT PROVISIONS OF ASTM D5456 AND

SPECIFICATIONS. ALL MEMBERS SHALL NOT HAVE NOTCHES OR DRILLED HOLES WITHOUT PRIOR ENGINEER

SHALL BE THE SIZE AND TYPE SHOWN ON THE DRAWINGS AS MANUFACTURED BY ILEVEL TRUS JOIST OR

COMBINATION SYMBOL

24F-V4

24F-V8

24F-V8

DRAWINGS AND PLANS. REFERENCE BCSI-B3 FOR TYPICAL PERMANENT BRACING REQUIREMENTS U.N.O.

08100 - EPOXY ADHESIVE ANCHORS EPOXY SPECIFIED IN THE DRAWINGS SHALL BE SIMPSON STRONG-TIE SET-XP EPOXY ADHESIVE. ANCHOR ROD, THREADED ROD, OR REINFORCING DIAMETER AND EMBEDMENT PER PLAN. INSTALLATION

08300 - SCREW ANCHORS

08200 - EXPANSION ANCHORS

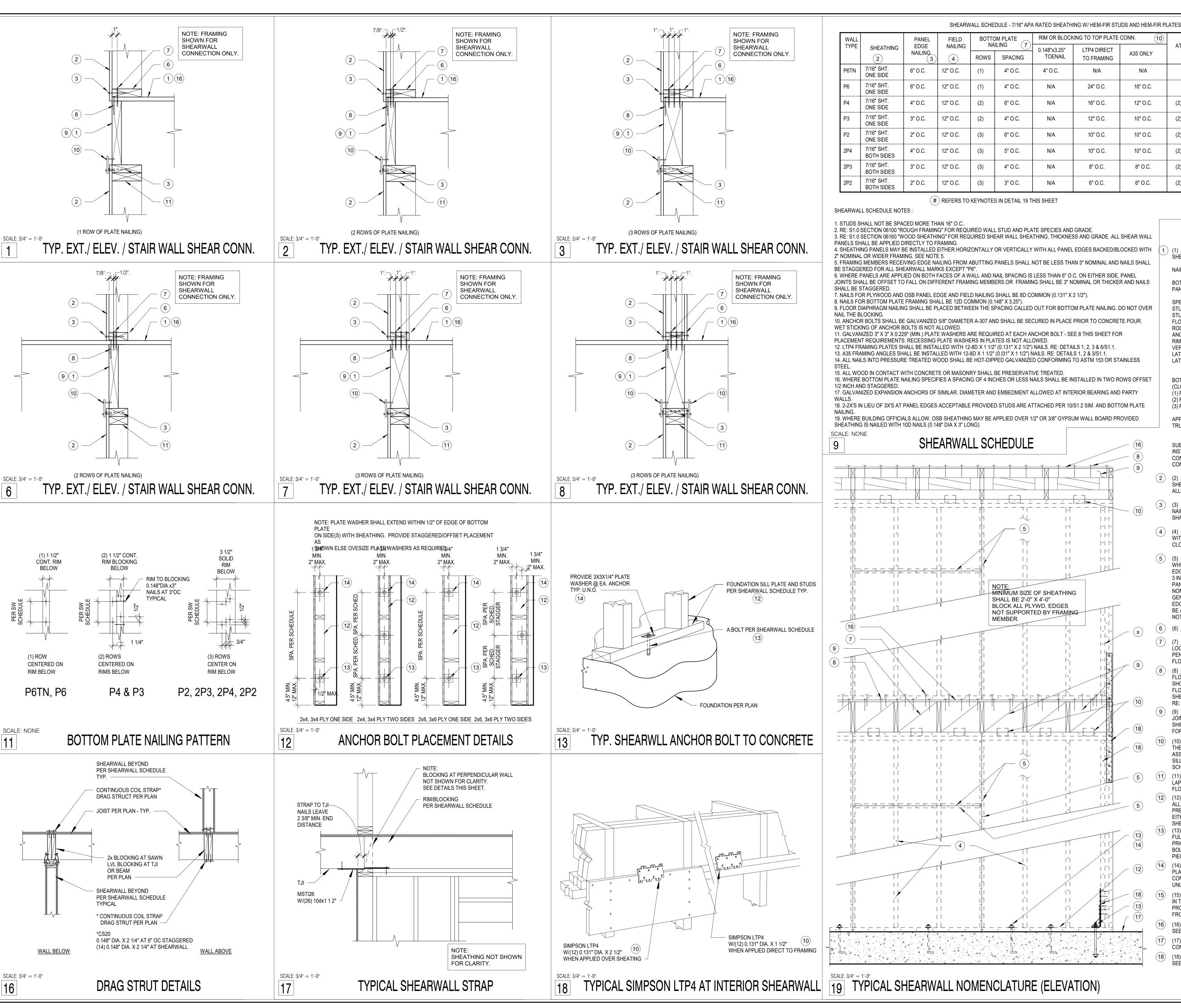
PER ESR-2508.

STRONG-BOLT WEDGE ANCHOR. ANCHOR DIAMETER AND EMBEDMENT PER PLAN. INSTALLATION PER SECTION 4.3 OF ESR-1771.

ANCHOR DIAMETER AND EMBEDMENT PER PLAN. INSTALLATION PER ESR-2713.

SCREW ANCHORS SPECIFIED IN THE DRAWINGS SHALL BE SIMPSON STRONG-TIE TITEN HD.

EXPANSION ANCHORS SPECIFIED IN THE DRAWINGS SHALL BE SIMPSON STRONG-TIE



RIM OR BLOCKING TO TOP PLATE CONN. FOUNDATION ANCHOR BOLT AT ADJOINING SILL PLATE A35 ONLY EDGES (5) 7" EMBED (48" O.C. 48" O.C. 16" O.C. 32" O.C. 12" O.C. (2)2x OR 3x 24" O.C. 10" O.C. (2)2x OR 3x 18" O.C. 10" O.C. (2)2x OR 3x 16" O.C. 10" O.C. (2)2x OR 3x 12" O.C. 8" O.C. (2)2x OR 3x 8" O.C. 6" O.C. (2)2x OR 3x

3. RE: S1.0 SECTION 06160 "WOOD SHEATHING" FOR REQUIRED SHEAR WALL SHEATHING, THICKNESS AND GRADE. ALL SHEAR WALL

6. WHERE PANELS ARE APPLIED ON BOTH FACES OF A WALL AND NAIL SPACING IS LESS THAN 6" O.C. ON EITHER SIDE, PANEL JOINTS SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS OR FRAMING SHALL BE 3" NOMINAL OR THICKER AND NAILS

9. FLOOR DIAPHRAGM NAILING SHALL BE PLACED BETWEEN THE SPACING CALLED OUT FOR BOTTOM PLATE NAILING. DO NOT OVER

11. GALVANIZED 3" X 3" X 0.229" (MIN.) PLATE WASHERS ARE REQUIRED AT EACH ANCHOR BOLT - SEE 8 THIS SHEET FOR

14. ALL NAILS INTO PRESSURE TREATED WOOD SHALL BE HOT-DIPPED GALVANIZED CONFORMING TO ASTM 153 OR STAINLESS

17. GALVANIZED EXPANSION ANCHORS OF SIMILAR. DIAMETER AND EMBEDMENT ALLOWED AT INTERIOR BEARING AND PARTY

19. WHERE BUILDING OFFICIALS ALLOW, OSB SHEATHING MAY BE APPLIED OVER 1/2" OR 3/8" GYPSUM WALL BOARD PROVIDED

SHEATHING: 7/16" CD-CC SHEATHING

APPLIED DIRECTLY TO FRAMING USE LENGTH DIA. BOTTOM PLATE/FRAMING 3 1/4" X 0.148" PANEL EDGE NAILING 2 3/8" X 0.148"

STUD SPACING: 16"O.C. MAX. STUDS AND PLATE: HEM-FIR #2 OR BETTER FLOOR THICKNESS: 23/32"

ROOF THICKNESS: ANCHOR BOLT: RIM/BLOCKING: 0.148" DIA. NAILS AT 4" O.C./SG=0.50 VERTICAL LOAD TRANSFER CAPACITY3300 LB./FT. LATERAL LOAD TRANSFER CAPACITY (1.25") 600 LB./FT. LATERAL LOAD TRANSFER CAPACITY (3.50") 1200 LB./FT.

BOTTOM PLATE NAILING NO. PIECES/THICKNESS (CLOSEST SPACING) (1) ROWS 0.148" DIA. AT 4" O.C.(1) / 1.25 (2) ROWS 0.148" DIA. AT 4" O.C.(1) / 1.75"

(3) ROWS 0.148" DIA. AT 4" O.C.(1) / 3.50" APPROVED RIM PRODUCTS:

TRUS JOIST ER-4979 TIMBERSTRAND LSL 2.0E, PARALLAM PSL 2.0E TJ-STRAND, MICROLAM LVL RIM BOARD

SUBSTITUTIONS TO ABOVE REQUIRE ENGINEER OF RECORD APPROVAL PRIOR TO INSTALLATION. SUBMIT DOCUMENTATION BY A CODE APPROVED AGENCY. CONFIRMING THE REQUIRED CAPACITIES AND MINIMUM NAIL SPACING FOR THE

SHEATHING PANELS MAY BE INSTALLED EITHER VERTICALLY OR HORIZONTALLY. ALL PANEL EDGES SHALL BE FASTENED TO STUDS OR BLOCKING.

(3) <u>PANEL EDGE NAILING:</u>
NAILING AT ALL OUTER EDGES OF SHEATHING PANELS IN SHEARWALLS SHALL BE FASTENED PER THE SHEARWALL SCHEDULE.

WITHIN THE FIELD OF THE PANEL, AT FRAMING MEMBERS, THE PANELS ARE LESS

CLOSELY FASTENED.

WHERE TWO PIECES OF PLYWOOD JOIN ON A FRAMING MEMBER, THE PANEL EDGE NAILING FROM EACH PANEL IS TO BE STAGGERED. SOME WALLS REQUIRE 3 INCH NOMINAL FRAMING MEMBER (EITHER A STUD OR BLOCKING) AT ADJOINING PANEL EDGES (SEE SHEARWALL SCHEDULE FOR WALL TYPES REQUIRING 3 INCH NOMINAL FRAMING MEMBERS AT ADJOINING PANEL EDGES). DOUBLED STUDS ARE GENERALLY NOT ACCEPTABLE FOR THIS APPLICATION. WHERE A SINGLE PANEL EDGE LANDS ON A FRAMING MEMBER, A 2 INCH NOMINAL FRAMING MEMBER SHALI BE ACCEPTABLE (AT ENDS OF WALLS FOR EXAMPLE). BLOCK ALL PLYWOOD EDGES NOT SUPPORTED BY FRAMING MEMBERS AND NAIL W/PANEL EDGE NAILING.

OCATE THE NAILING THROUGH THE BOTTOM PLATE SO AS TO FULLY PENETRATE THE SOLID BLOCKING OR CONTINUOUS RIM BENEATH THE FLOOR SHEATHING, SPACED AS PER THE SHEARWALL SCHEDULE. (8) ROOF DIAPHRAGM BOUNDAY EDGE NAILING

FLOOR DIAPHRAGM NAILING SHALL BE INSTALLED BETWEEN THE SPACING SHOWN FOR BOTTOM PLATE NAILING. LOCATE ADJOINING PANEL EDGES OF FLOOR SHEATHING AWAY FROM SHEARWALLS. FIELD NAILING OF FLOOR SHEATHING MAY BE OMITTED AT SHEARWALL BOTTOM PLATE NAILING.

JOIN ADJACENT TRUSS BLOCKING PANEL WITH FACE NAILING AS SPECIFIED ABOVE SHIM WITH FULL HEIGHT SHIMS, ADJUST FACE NAIL LENGTHS. REFER TO PLANS FOR ADDITIONAL SEISMIC CONNECTIONS AT THE FLOOR OR ROOF LEVEL.

(10) TRUSS BLOCKING PANEL TO TOP PLATE CONNECTION: THE CONTINUOUS TRUSS BLOCKING PANEL THAT IS PART OF THE SHEARWALL ASSEMBLY SHALL BE CONNECTED TO THE DOUBLE TOP PLATE OR FOUNDATION SILL PLATE WITH APPROVED CONNECTORS AND SPACED PER THE SHEARWALL

LAP AND SPLICE - SEE PLANS FOR ADDITIONAL SEISMIC CONNECTIONS AT THE FLOOR OR ROOF LEVEL.

ALL WOOD IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESERVATIVE TREATED. THE FOUNDATION SILL PLATE SHALL BE EITHER 2 INCH NOMINAL OR 3 INCH NOMINAL DEPENDING ON THE

FULL DIAMETER ANCHOR BOLTS, ASTM A-307 SHALL BE SECURED IN PLACE PRIOR TO PLACING CONCRETE. MINIMUM EMBEDMENT IS 7 INCHES. MIN. (2) BOLTS PER PIECE OF PLATE, W/(1) BOLT NOT MORE THAN 12" FROM END OF

PLATE WASHERS SHALL BE REQUIRED FOR FOUNDATION SILL PLATE CONNECTIONS, 3" X 3" X 1/4" MINIMUM. DO NOT RECESS BOLTS IN SILL PLATE UNLESS SPECIFICALLY DETAILED ELSEWHERE.

IN THE FLOOR CAVITY OF PLATFORM FRAMING POST LOADS SHALL BE PROVIDED WITH ADDITIONAL STIFFENERS EQUAL TO THE POST SIZE FROM ABOVE THAT CONTINUES THROUGH THE FLOOR.

SEE (1) FOR SHEARWALL, FLOOR AND ROOF DIAPHRAGM THICKNESS.

CONCRETE FOUNDATION OR BASE

SEÉ SHEET S1.2 FOR HOLDOWN DETAILS AND ADDITIONAL STUDS REQUIRED.

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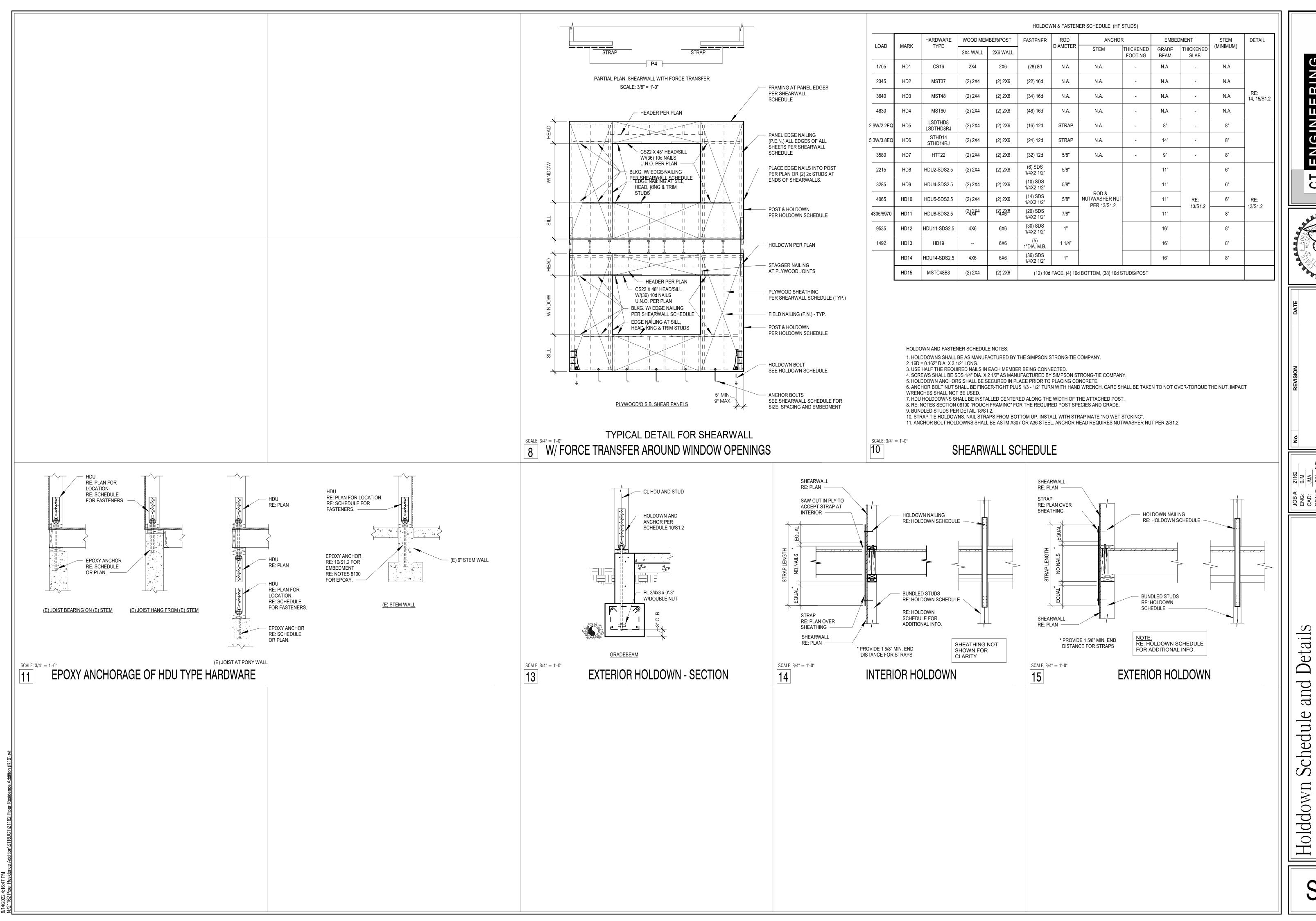
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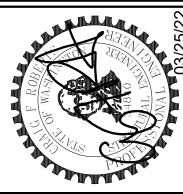
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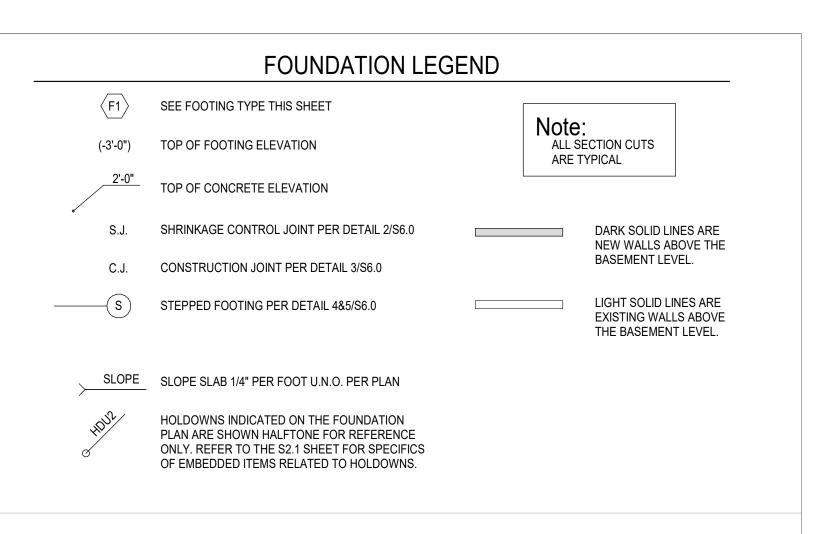
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PIPER REMODEL 8429 SE 33RD PLACE MERCER ISLAND, WA 99



FOUNDATION SCHEDULE

MARK	DEPTH	WIDTH	LENGTH	REINFORCING	DETAILS
F1	8"	1'-4"	CONT.	(2) #4 CONT.	FTG. W/ STEM WALL: 6&7/S6.0
FS	12"	1'-6"	CONT.	(2) #4 CONT.	TYP. THICKENED SLAB FOOTING
(F24)	12"	24"	24"	(2) #4 EA. WAY	POST FTG.: 9/S6.0 16&17/S6.0
(F30)	12"	30"	30"	(3) #4 EA. WAY	POST FTG.: 9/S6.0 16&17/S6.0
F36	12"	36"	36"	(3) #4 EA. WAY	POST FTG.: 9/S6.0 16&17/S6.0
⟨F1⟩	8"	1'-4"	CONT.	(2) #4 CONT.	TURNED DOWN SLAB EDGE 6,7,8/S6.0

FOUNDATION NOTES

1. ALL SOIL BEARING SURFACES ARE SUBJECT TO INSPECTION AND APPROVAL BY THE

GEOTECHNICAL ENGINEER PRIOR TO REINFORCING AND CONCRETE PLACEMENT. 2. CENTER INTERIOR FOOTINGS ON WALLS OR COLUMNS TYPICAL U.N.O.

3. VERIFY ALL DIMENSIONS PRIOR TO CONSTRUCTION.

4. SEE ARCHITECTURAL PLANS FOR WALL LOCATIONS. 5. CONCRETE WALLS ARE 8" THICK TYPICAL U.N.O.

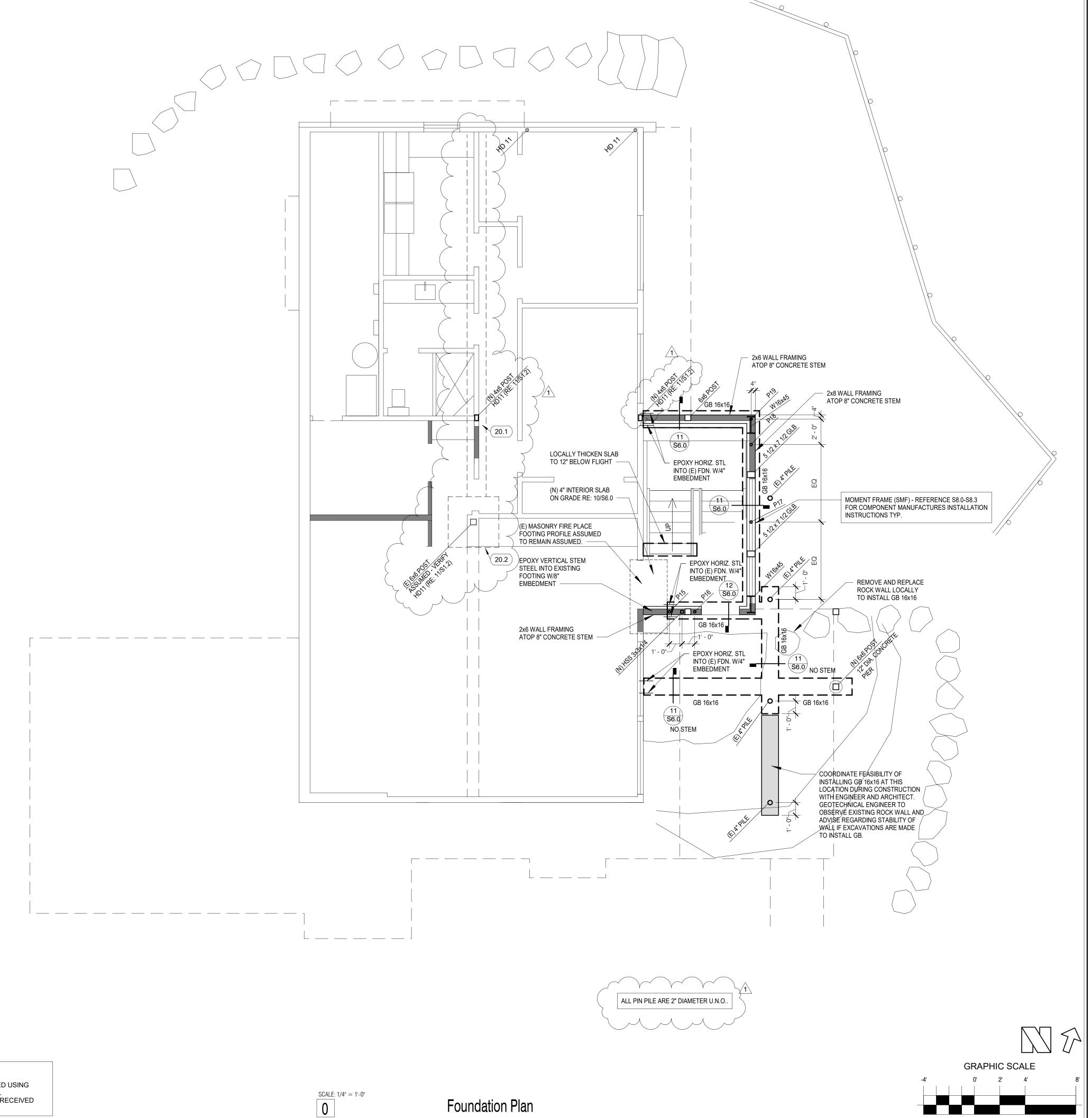
6. SEE SHEET S2.1 FOR WOOD FRAMING LEGEND, NOTES, AND SCHEDULES.

7. PROVIDE 4" DIAMETER PERFERATED FOOTING DRAINS AT PERIMETER OF FOUNDATIONS TYPICAL. PROVIDE 4" DIAMETER TIGHTLINES FOR DOWNSPOUTS, EXTEND TO DAYLIGHT.

FOUNDATION KEY NOTES

ASSUMED EXISTING STRIP FOOTING. CONTRACTOR TO VERIFY AND INFORM ENGINEER OF EXISTING CONDITIONS DURING CONSTRUCTION.

ASSUMED EXISTING PAD FOOTING. CONTRACTOR TO VERIFY AND INFORM ENGINEER OF EXISTING CONDITIONS DURING CONSTRUCTION.



PLANS PREPARED USING ARCHITECTURAL BACKGROUNDS RECEIVED 02/28/2022.

Foundation Plan

Basement

(1 inch = 4 foot)

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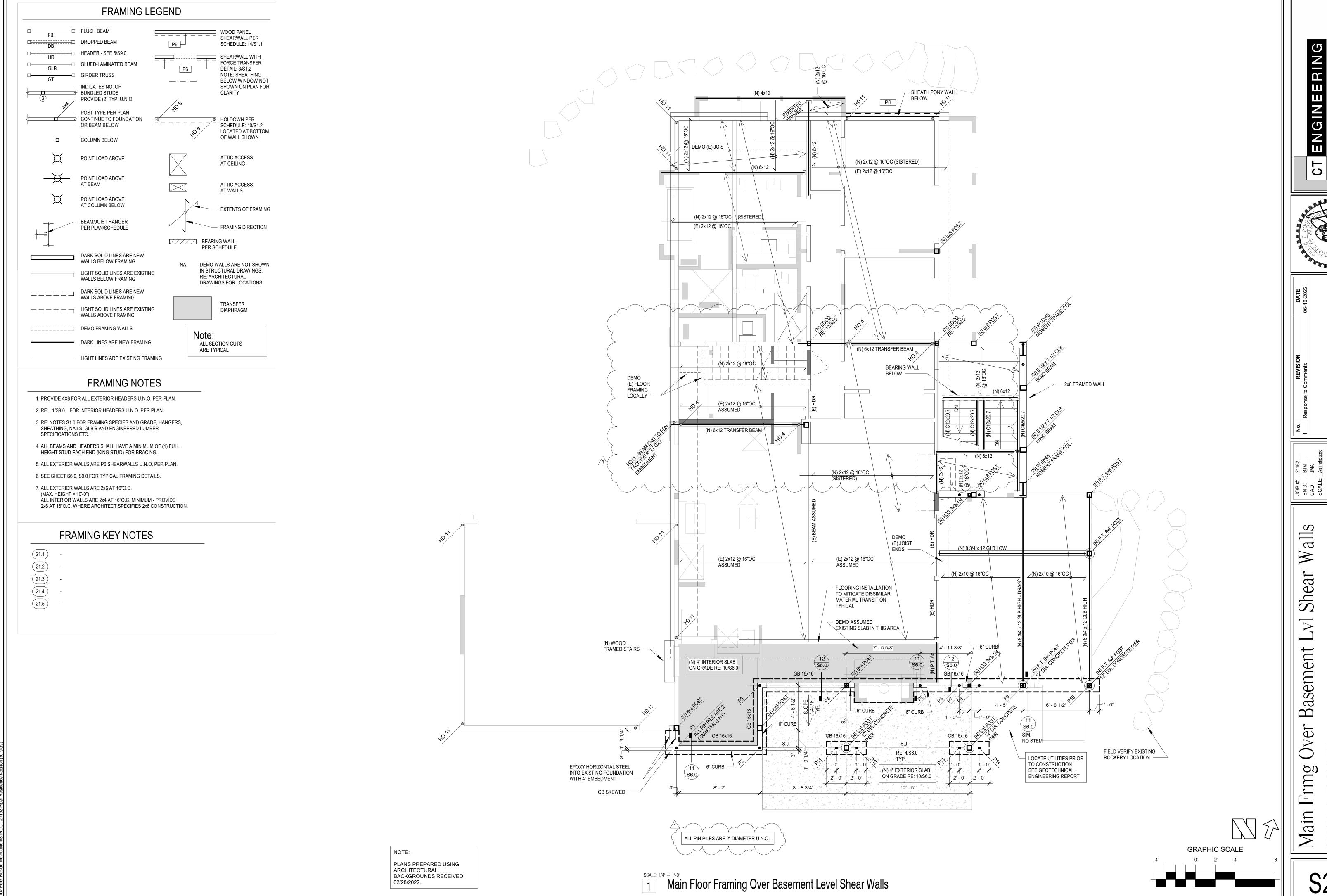
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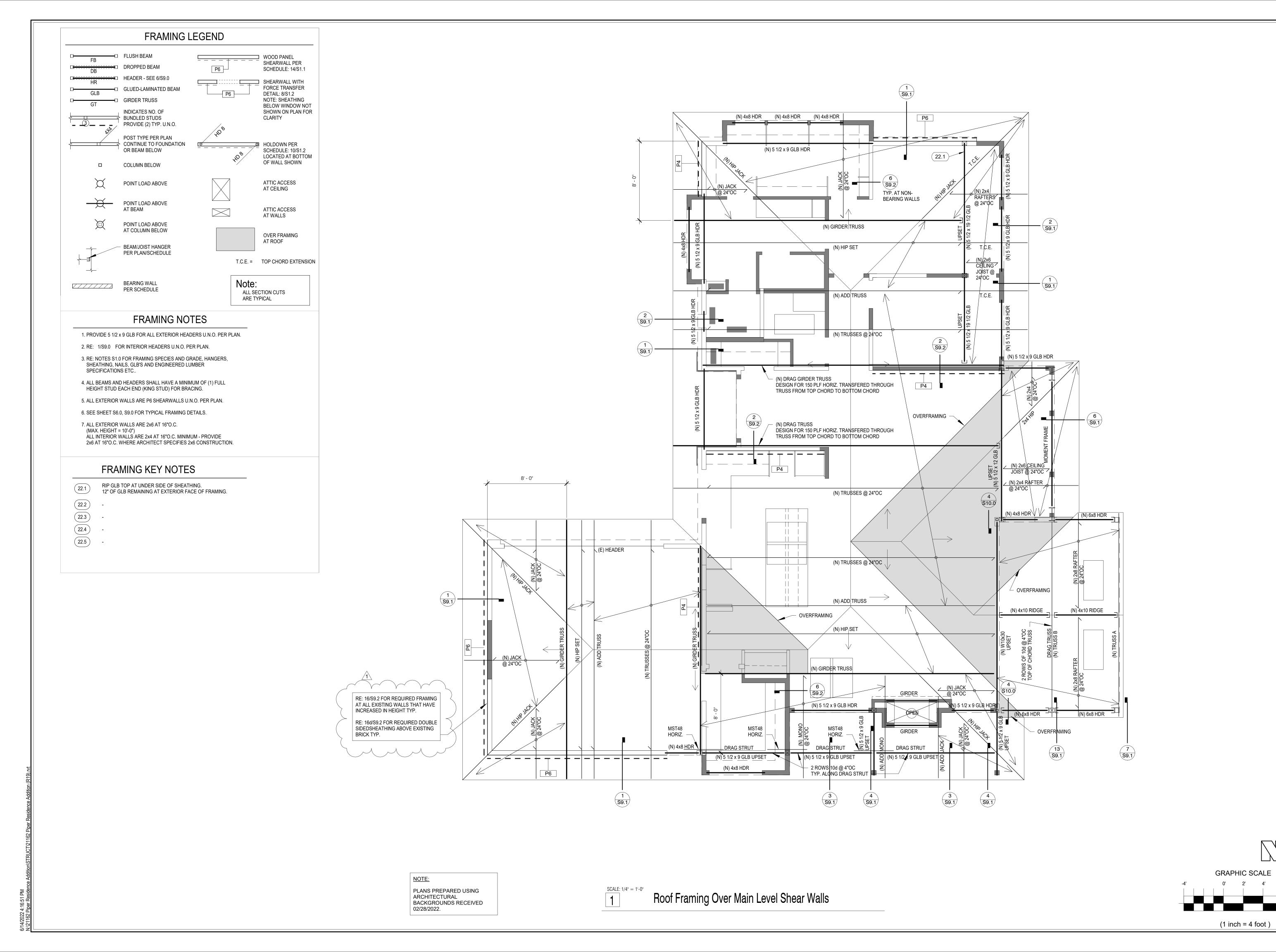
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Basement Over Main

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(1 inch = 4 foot)



Roof Framing Over Main Level Shear PIPER REMODEL
8429 SE 33RD PLACE
MERCER ISLAND, WA 98040

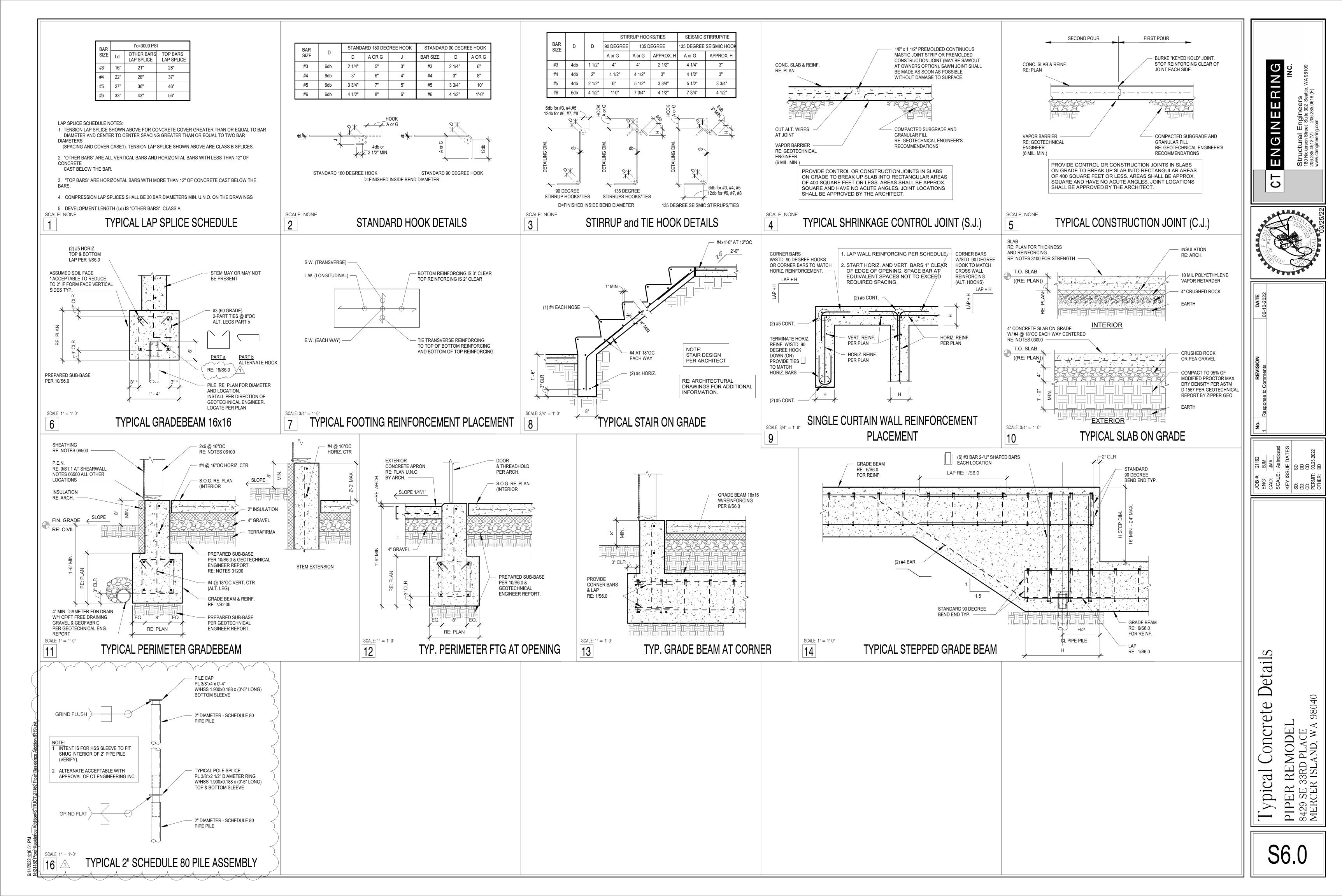
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S2.2



Simpson Strong—Tie[®] Strong Frame[®] and the Yield—Link[™] structural fuse are protected under one or more of the following US patents and applications: US patent

No. 8,001,734 B2, US patent No. 8,375,652 B2, and US patent publication No. 2015/0159362, and must be supplied or licensed through Simpson Strong—tie.

- PROVIDE 1160 LBS (ASD) FOR TOP

15'-9"

OF COLUMN STABILITY BRACING, TYP.

1. SIMPSON STRONG-TIE® STRONG FRAME® AND THE YIELD-LINK™ STRUCTURAL FUSE ARE PROTECTED UNDER ONE OR MORE OF THE FOLLOWING US PATENTS AND APPLICATIONS: US PATENT NO. 8,001,734 B2, US PATENT NO. 8,375,652 B2, AND US PATENT PUBLICATION NO. 2015/0159362, AND MUST BE

SUPPLIED OR LICENSED THROUGH SIMPSON STRONG-TIE. 2. STRONG FRAME® SPECIAL MOMENT FRAME IS MANUFACTURED AND TRADEMARKED BY "SIMPSON STRONG-TIE COMPANY INC." HOME OFFICE: 5956 W. LAS POSITAS BLVD., PLEASANTON, CA 94588 TEL: (800) 999-5099, FAX: (925) 847-1597. "SIMPSON STRONG-TIE COMPANY INC." IS AN ISO 9001 REGISTERED COMPANY.

3. DESIGN FOR STRONG FRAME® MOMENT FRAMES ARE IN ACCORDANCE WITH THE FOLLOWING:

- 2018, 2015 AND 2012 INTERNATIONAL BUILDING CODE

- AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS (ANSI/AISC 360-05, 360-10, 360-16)

- AISC SEISMIC PROVISIONS (ANSI/AISC 341-05, 341-10, 341-16)

- RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS - BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI318-11, ACI318-14)

4. USE OF THIS PRODUCT IS SUBJECT TO THE APPROVAL OF THE LOCAL BUILDING DEPARTMENT. 5. THIS PRODUCT IS PART OF THE OVERALL LATERAL FORCE RESISTING SYSTEM OF THE STRUCTURE. DESIGN OF THE BUILDING'S LATERAL FORCE RESISTING SYSTEM, INCLUDING THE LOAD PATH TO TRANSFER LATERAL FORCES FROM THE STRUCTURE TO THE GROUND, IS THE RESPONSIBILITY OF THE

6. THE DESIGNER MUST SPECIFY THE REQUIRED COMPONENTS OF THE COMPLETE LOAD TRANSFER PATH INCLUDING DIAPHRAGMS, SHEAR TRANSFER,

CHORDS AND COLLECTORS AND FOUNDATIONS 7. ALL CONNECTED MEMBERS AND RELATED ELEMENTS SHALL BE DESIGNED BY THE DESIGNER.

8. DESIGNER IS PERMITTED TO MODIFY DETAILS FOR SPECIFIC CONDITIONS. SEE LIMITATIONS NOTED ON SHEET SMF3.

9. ANCHORAGE LENGTHS PROVIDED ARE SHOWN FOR MINIMUM EMBEDMENT INTO FOOTING BASED ON TENSION ANCHORAGE DESIGN ONLY. ACTUAL LENGTH OF ANCHORAGE SHALL BE PER DESIGNER'S SPECIFICATIONS AND PROJECT SPECIFIC INSTALLATION REQUIREMENTS.

10. PRE-ASSEMBLED ANCHORAGE KITS PROVIDED BY SIMPSON (MFSL OR MFAB) SHALL BE SPECIFIED BY DESIGNER AND SHOULD INCLUDE ANCHORAGE TYPE, ROD GRADE, AND LENGTH OF ASSEMBLY. REFER TO DETAIL 2 FOR AVAILABLE LENGTHS OF FULLY ASSEMBLED ANCHORAGE ASSEMBLIES.

EXTENSION KITS IN 36" LENGTHS ARE AVAILABLE FOR USE IN STEMWALLS OR APPLICATIONS WHERE DEEPER EMBEDMENT IS REQUIRED. 11. FOOTING DIMENSIONS SHOWN ARE THE MINIMUMS REQUIRED FOR CONCRETE ANCHORAGE REQUIREMENTS ONLY. THE DESIGNER MUST DETERMINE REQUIRED FOOTING SIZE AND REINFORCING FOR OTHER DESIGN LIMITS. SUCH AS FOUNDATION SHEAR AND BENDING. SOIL BEARING SHEAR TRANSFER. AND FRAME STABILITY / OVERTURNING.

12. DESIGNER MUST DETAIL ACTUAL FOOTING / GRADE BEAM SIZE AND REINFORCING.

13. HOLES IN BASE PLATES ARE OVER-SIZED FOR ERECTION TOLERANCE. DESIGNER MUST EVALUATE EFFECTS OF OVER-SIZED HOLES AND PROVIDE PLATE WASHER WITH STANDARD-SIZE HOLES WELDED TO BASE PLATE OR REQUEST BASE PLATES WITH STANDARD SIZE HOLES WHERE REQUIRED.

14. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, CONDITIONS, ELEVATIONS, ETC. PRIOR TO INSTALLATION OF ANY COMPONENTS FOR THE STEEL STRONG FRAME SYSTEM. IF ANY DISCREPANCIES ARE FOUND, THEY SHALL BE BROUGHT TO THE ATTENTION OF THE DESIGNER FOR CLARIFICATION PRIOR TO CONSTRUCTION.

15. INSTALLATION OF PRODUCT SHALL BE DONE IN CONFORMANCE WITH THESE DRAWINGS AND ICC ESR-2802. THE PERFORMANCE OF MODIFIED PRODUCTS OR ALTERED INSTALLATION PROCEDURES ARE THE SOLE RESPONSIBILITY OF THE DESIGNER.

16. SIMPSON STRONG-TIE® COMPANY, INC. RESERVES THE RIGHT TO CHANGE SPECIFICATIONS, DESIGNS, AND MODELS WITHOUT NOTICE OR LIABILITY FOR

SUCH CHANGES. 17. ALL HARDWARE CALLED OUT IS SIMPSON STRONG-TIE[®].

18. USE OF A SIMPSON STRONG—TIE PRODUCT DOES NOT IMPLY THAT SIMPSON STRONG—TIE ENDORSES ANY PROJECT, STRUCTURE OR USE. NO LICENSE IS GRANTED WITH RESPECT TO ANY SIMPSON STRONG-TIE TRADEMARK OR OTHER INTELLECTUAL PROPERTY RIGHTS. WRITTEN PERMISSION MUST BE OBTAINED PRIOR TO USING ANY SIMPSON STRONG-TIE TRADEMARKS OR PROPRIETARY DOCUMENTS AND MATERIALS

19. SIMPSON STRONG-TIE IS NOT AFFILIATED WITH, AND DOES NOT SPONSOR OR ENDORSE, THE DESIGNER, INSTALLER OR USERS OF THIS DRAWING, NOR DOES SIMPSON STRONG-TIE HAVE ANY JOINT VENTURE, PARTNERSHIP, AGENCY, EMPLOYMENT OR FIDUCIARY RELATIONSHIP WITH SUCH PERSONS.

1. BARS/PLATES: ASTM 572 GR. 50, ASTM A529 GR. 50, OR ASTM A1011 HSLAS GR. 50

2. W-SECTIONS (HOT ROLLED SECTIONS): ASTM A992

3. LINK TO COLUMN FLANGE HIGH STRENGTH BOLTS: 7/8" DIA. ASTM A325, TYPE 1 (SNUG-TIGHT)

4. BRP TO BEAM FLANGE AND SHEAR PLATE TO BEAM WEB HIGH STRENGTH BOLTS: ASTM A325, TYPE 1 (SNUG-TIGHT)

5. LINK TO BEAM FLANGE HIGH STRENGTH BOLTS: ASTM F2280 TWIST OFF TYPE (A490 EQUIVALENT) (PRETENSIONED) 6. BEAM TOP FLANGE WOOD NAILER BOLT: ASTM A307 GR. A

7. CARRIAGE BOLTS: ASTM A307 GR. A

8. ANCHOR RODS: ASTM F1554 GR 36 OR A36 (MFAB, MFSL, AND MF-ATR6EXT-LS); ASTM A449 (MFAB-HS, MFSL-HS, AND MF-ATR6EXT-HS)

9. GROUT: ASTM C1107, MINIMUM 5,000 PSI COMPRESSIVE STRENGTH

INSTALLATION AND FIELD MODIFICATIONS

THESE GENERAL INSTRUCTIONS FOR THE INSTALLER ARE PROVIDED TO ENSURE PROPER SELECTION AND INSTALLATION OF SIMPSON STRONG-TIE COMPANY INC. PRODUCTS AND MUST BE FOLLOWED CAREFULLY. THESE GENERAL INSTRUCTIONS ARE IN ADDITION TO THE SPECIFIC INSTALLATION INSTRUCTIONS AND NOTES PROVIDED FOR EACH PARTICULAR PRODUCT, ALL OF WHICH SHOULD BE CONSULTED PRIOR TO AND DURING INSTALLATION OF SIMPSON STRONG-TIE COMPANY INC. PRODUCTS.

1. PROPER PRODUCT INSTALLATION REQUIRES CAREFUL ATTENTION TO ALL NOTES AND INSTRUCTIONS. IN ADDITIONAL TO THE NOTES, WARNINGS, AND INSTRUCTIONS PROVIDED IN THE CATALOG, INSTALLERS, DESIGNERS, ENGINEERS AND CONSUMERS SHOULD CONSULT THE SIMPSON STRONG-TIE COMPANY INC. WEBSITE AT WWW.STRONGTIE.COM TO OBTAIN ADDITIONAL INFORMATION FOR INSTALLATION, SPECIFICATIONS, CODE REPORTS, TECHNICAL FLIERS AND

3. USE PROPER SAFETY AND INSTALLATION EQUIPMENT DURING INSTALLATION OF STRONG FRAME $^{\circ}$. 4. ALL SPECIFIED FASTENERS MUST BE INSTALLED ACCORDING TO THE INSTRUCTIONS PROVIDED IN THE CATALOG, CODE REPORT, AND INSTALLATION

DETAILS. INCORRECT FASTENER QUANTITY, SIZE, PLACEMENT, TYPE, MATERIAL, OR FINISH MAY CAUSE THE CONNECTION TO FAIL. 5. FILL ALL FASTENER HOLES AS SPECIFIED IN THE INSTALLATION INSTRUCTIONS FOR THE SPECIFIED PRODUCT, INSTALL ALL FASTENERS BEFORE LOADING

THE FRAME. SOME PRE-INSTALLED ITEMS MAY NOT USE ALL HOLES.

6. NUTS SHALL BE INSTALLED SUCH THAT THE END OF THE THREADED ROD OR BOLT IS AT LEAST FLUSH WITH THE TOP OF THE NUT.

REFER TO DETAIL 12/SMF3 FOR ALLOWABLE HOLE OPENINGS IN BEAM AND COLUMNS.

8. REFER TO DETAIL 11/SMF3 FOR CONNECTION PROTECTED ZONE.

9. WELDING SHALL BE IN ACCORDANCE WITH AWS D1.1 AND AWS D1.8 (AS APPLICABLE FOR SEISMIC). WELDS SHALL BE SPECIFIED BY THE DESIGNER.

PROVIDE WELDING SPECIAL INSPECTION AS REQUIRED BY THE LOCAL BUILDING DEPARTMENT.

1. WELDING OF FRAME MEMBERS AND APPLICABLE WELDING SPECIAL INSPECTIONS REQUIRED BY IBC SECTION 1707 ARE PERFORMED ON THE PREMISES

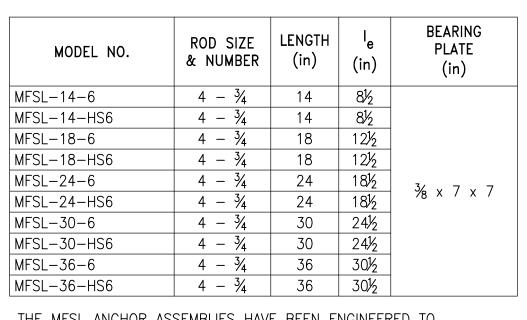
OF A FABRICATOR REGISTERED AND APPROVED IN ACCORDANCE WITH THE REQUIREMENTS OF IBC SECTION 1704.2.5 FOR FABRICATOR APPROVAL. 2. PRE-INSTALLATION VERIFICATION TESTING IS PERFORMED ON HIGH-STRENGTH FASTENER ASSEMBLIES.

3. INSPECTION REQUIREMENTS OUTSIDE THE SHOP MANUFACTURING AND ASSEMBLY PROCESS SHALL BE IN ACCORDANCE WITH THE LOCAL CODE, BASED ON BUILDING OCCUPANCY, CONCRETE STRENGTH, REQUIREMENTS OF THE LOCAL BUILDING OFFICIAL, AND OTHER CONSIDERATIONS AND SHALL BE SPECIFIED BY THE DESIGNER.

4. GROUTING UNDER COLUMN BASE PLATE MAY REQUIRE SPECIAL INSPECTION, CONTACT THE LOCAL BUILDING DEPARTMENT FOR COMPLIANCE

5. CONTACT SIMPSON STRONG-TIE® AT 800-999-5099 TO REQUEST PRE-INSTALLATION TESTING, WELDING REPORTS, MILL CERTS, ETC. WHEN REQUIRED.

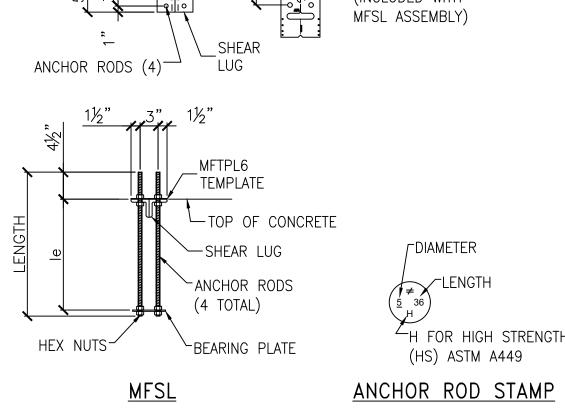
GENERAL NOTES



THE MFSL ANCHOR ASSEMBLIES HAVE BEEN ENGINEERED TO PROVIDE A COMPLETE ANCHORAGE SOLUTION MEETING THE 2012 AND 2015, 2018 INTERNATIONAL BUILDING CODE REQUIREMENTS FOR BOTH TENSION AND SHEAR.

ANCHOR RODS AND THE MFTPL TEMPLATE ARE INCLUDED PRE-ATTACHED WITH THE ASSEMBLY.

INSPECTION IS EASY; THE HEAD IS STAMPED WITH A "NO EQUAL" SYMBOL FOR IDENTIFICATION, BOLT LENGTH, BOLT DIAMETER, AND OPTIONAL "HS" FOR HIGH STRENGTH IF SPECIFIED.

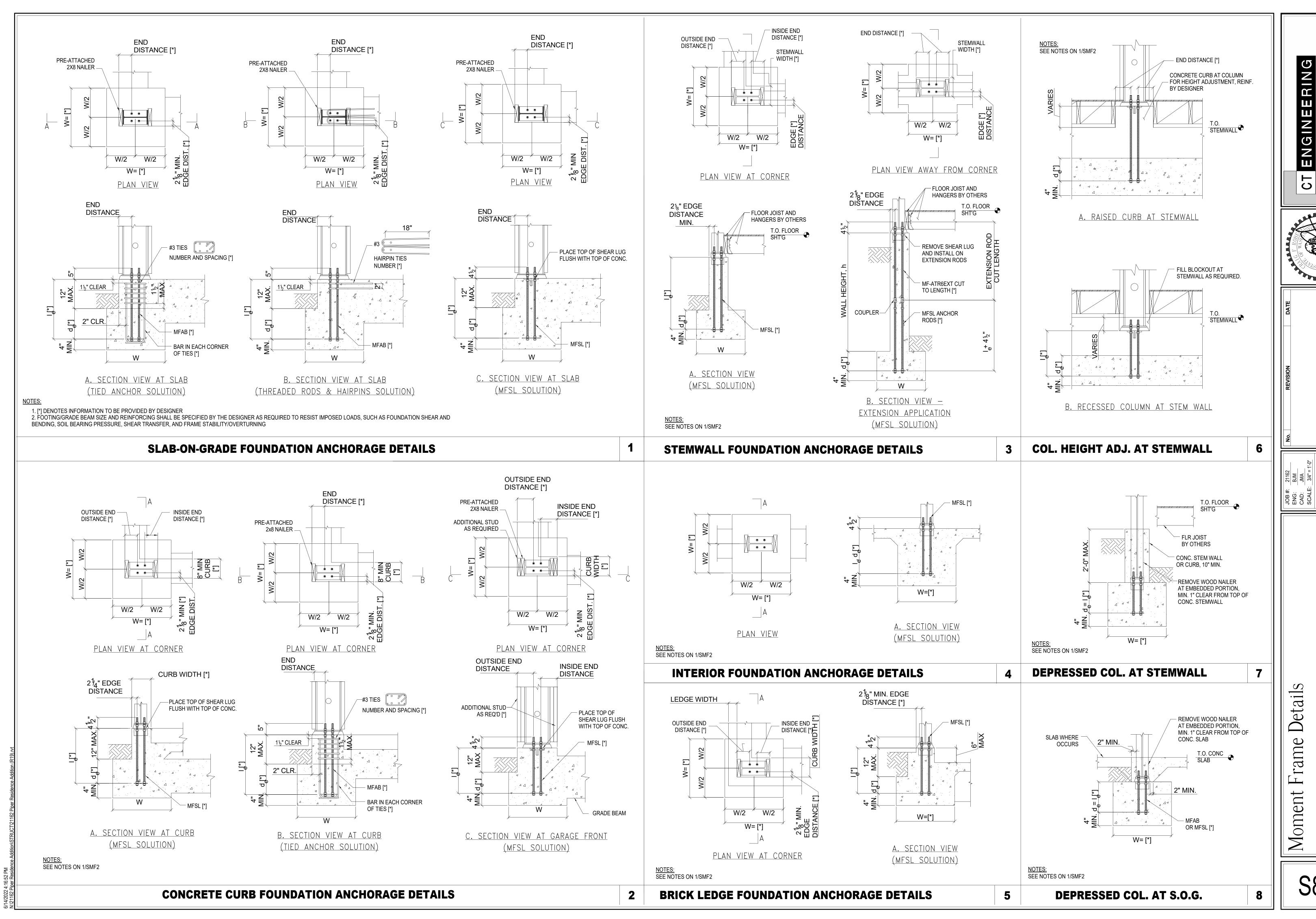


MFSL ANCHORAGE ASSEMBLIES

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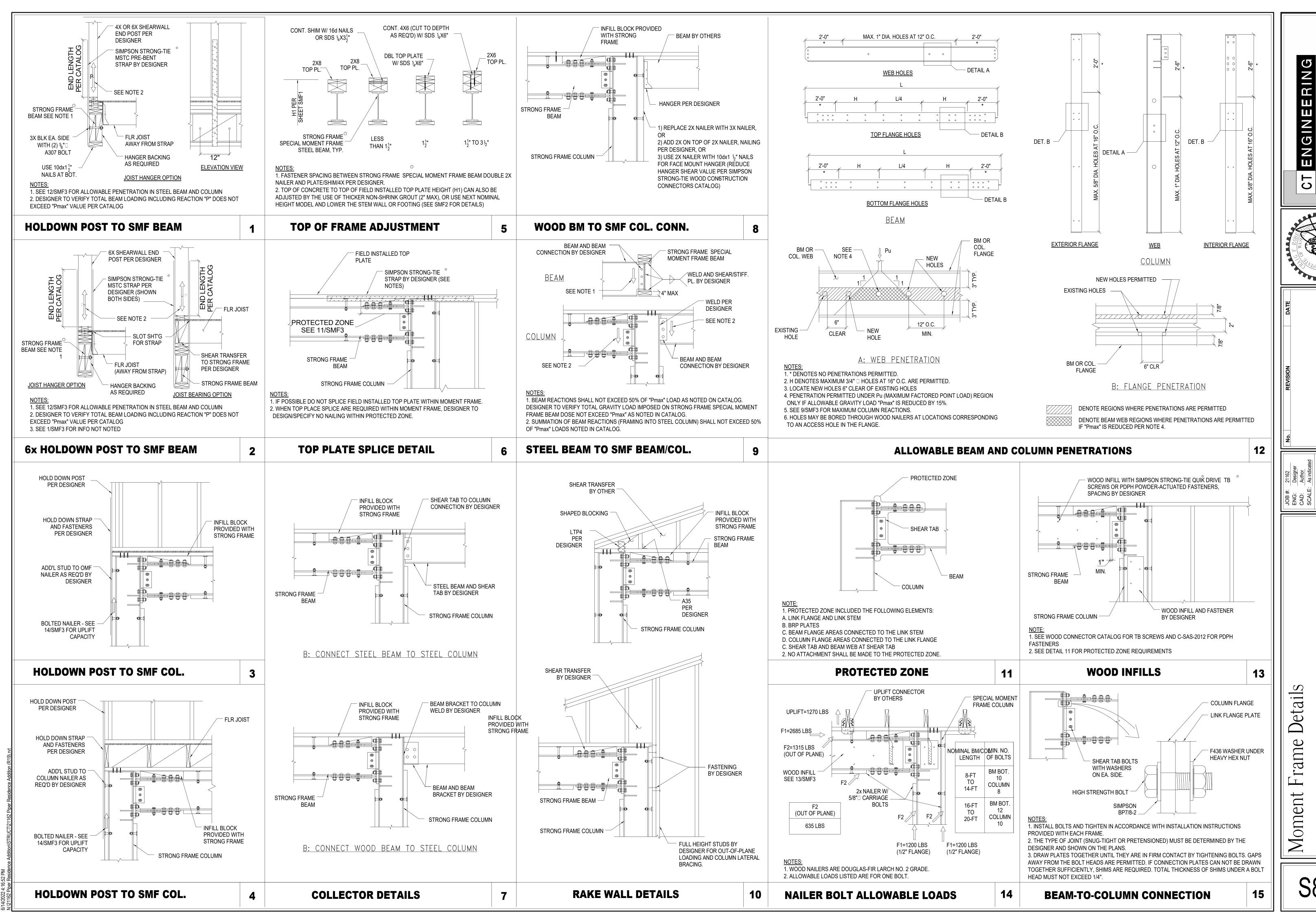
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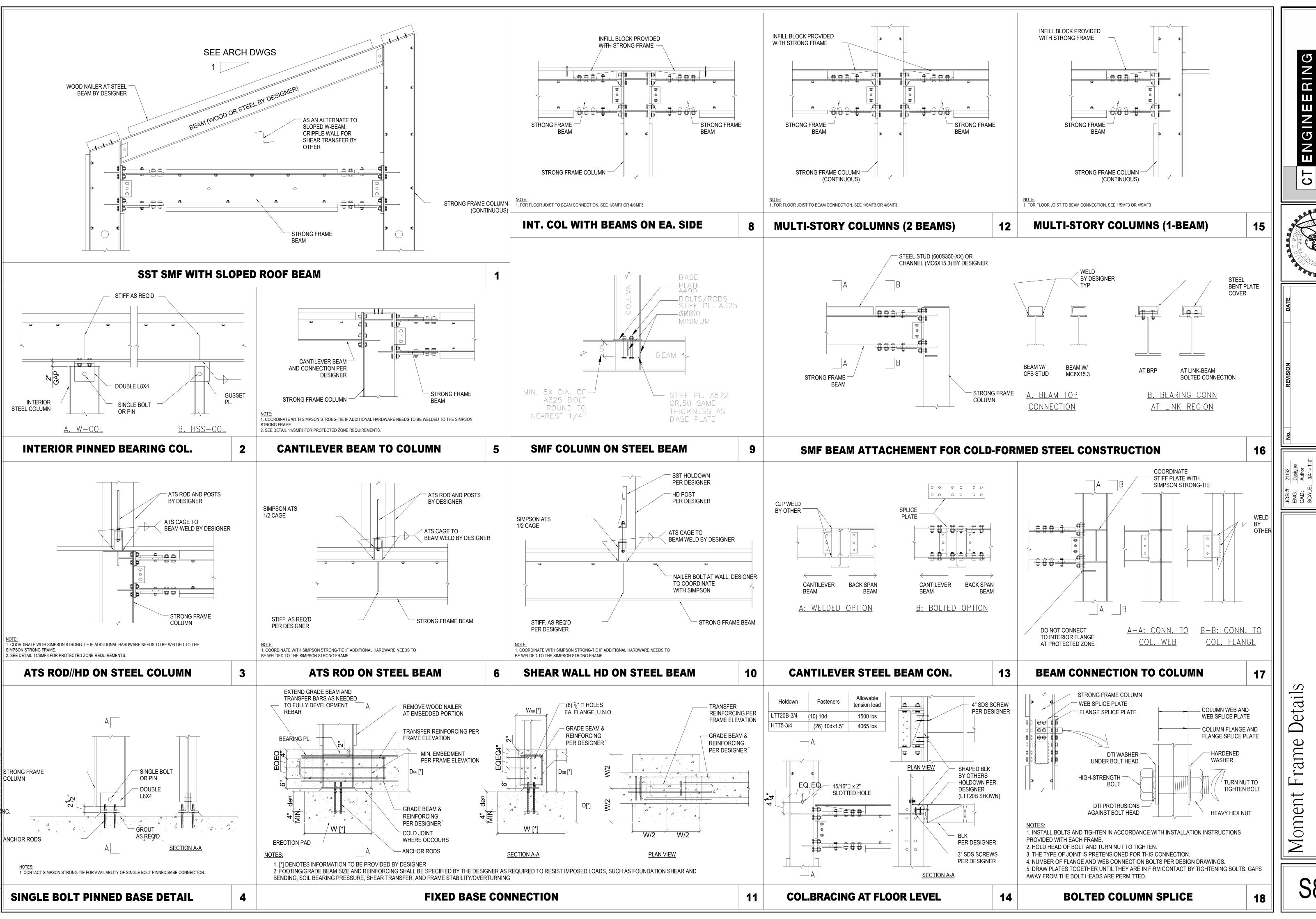
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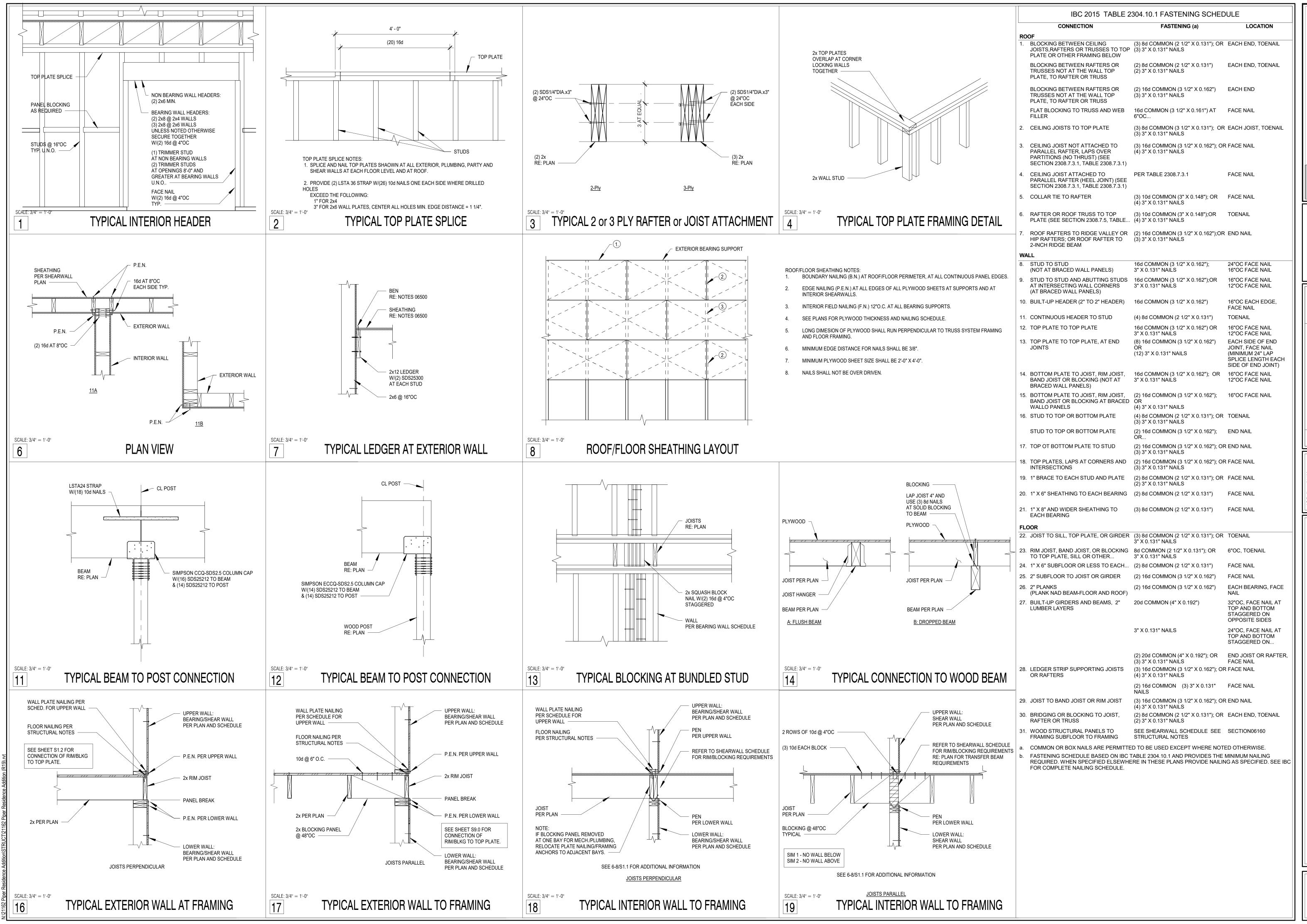
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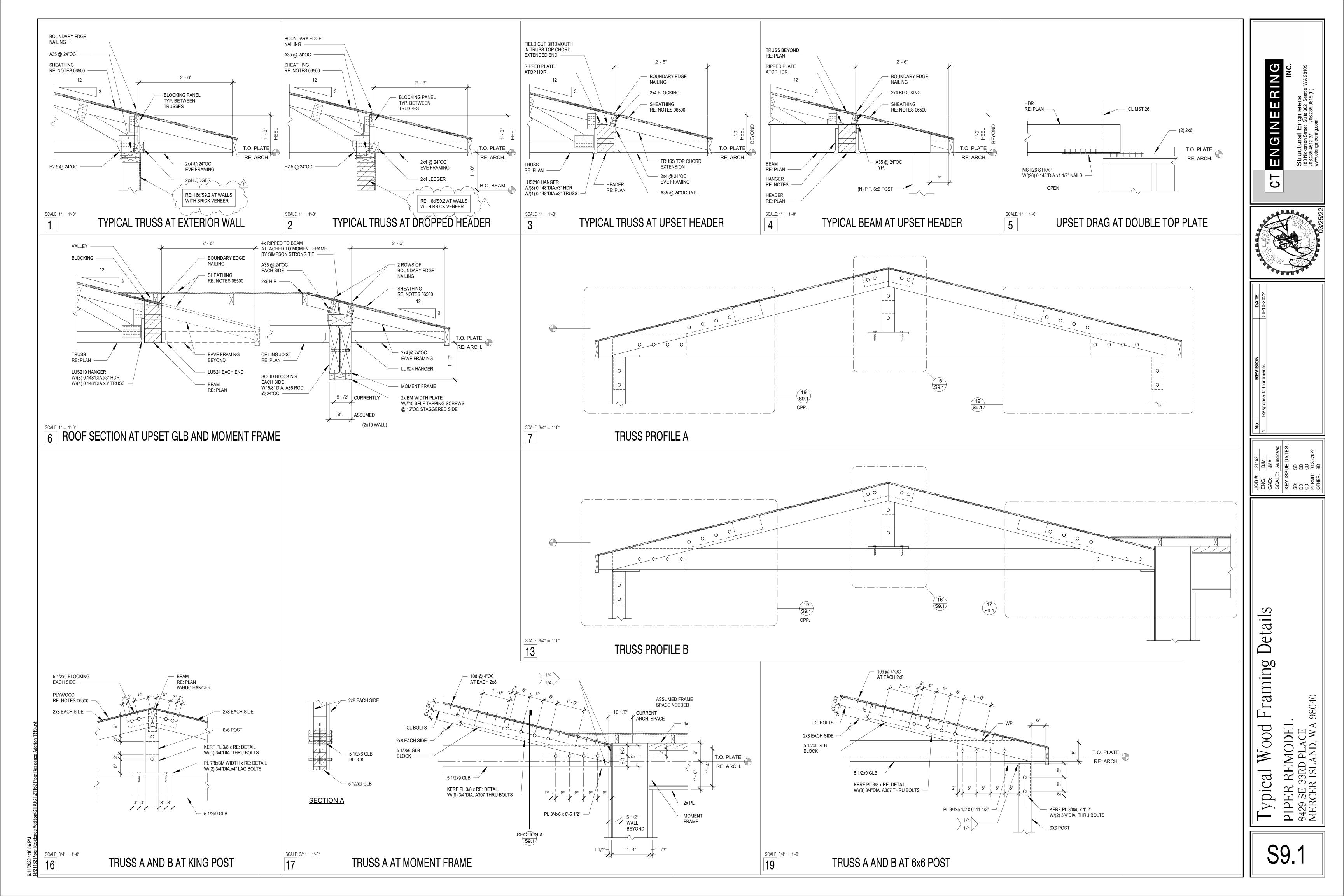
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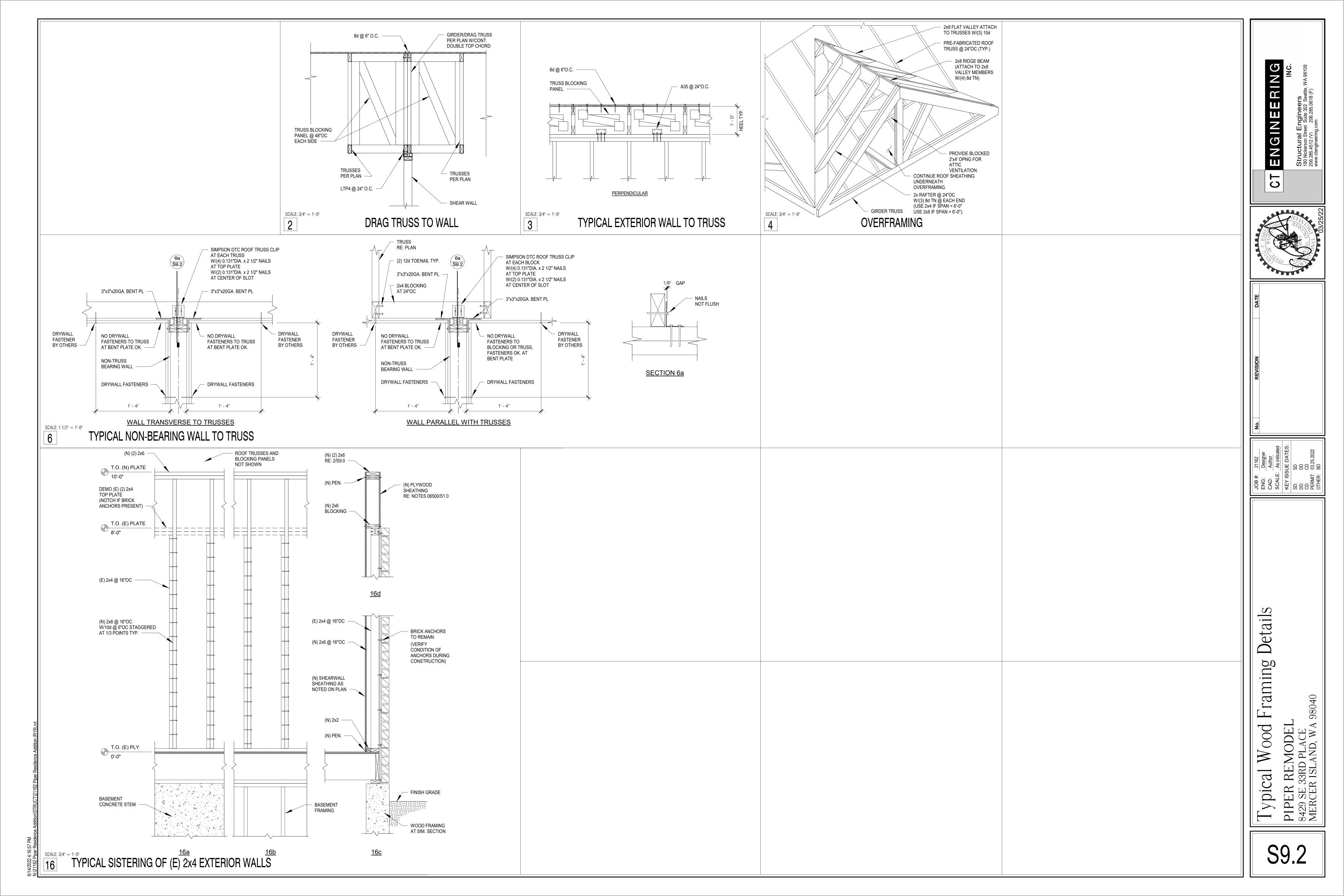


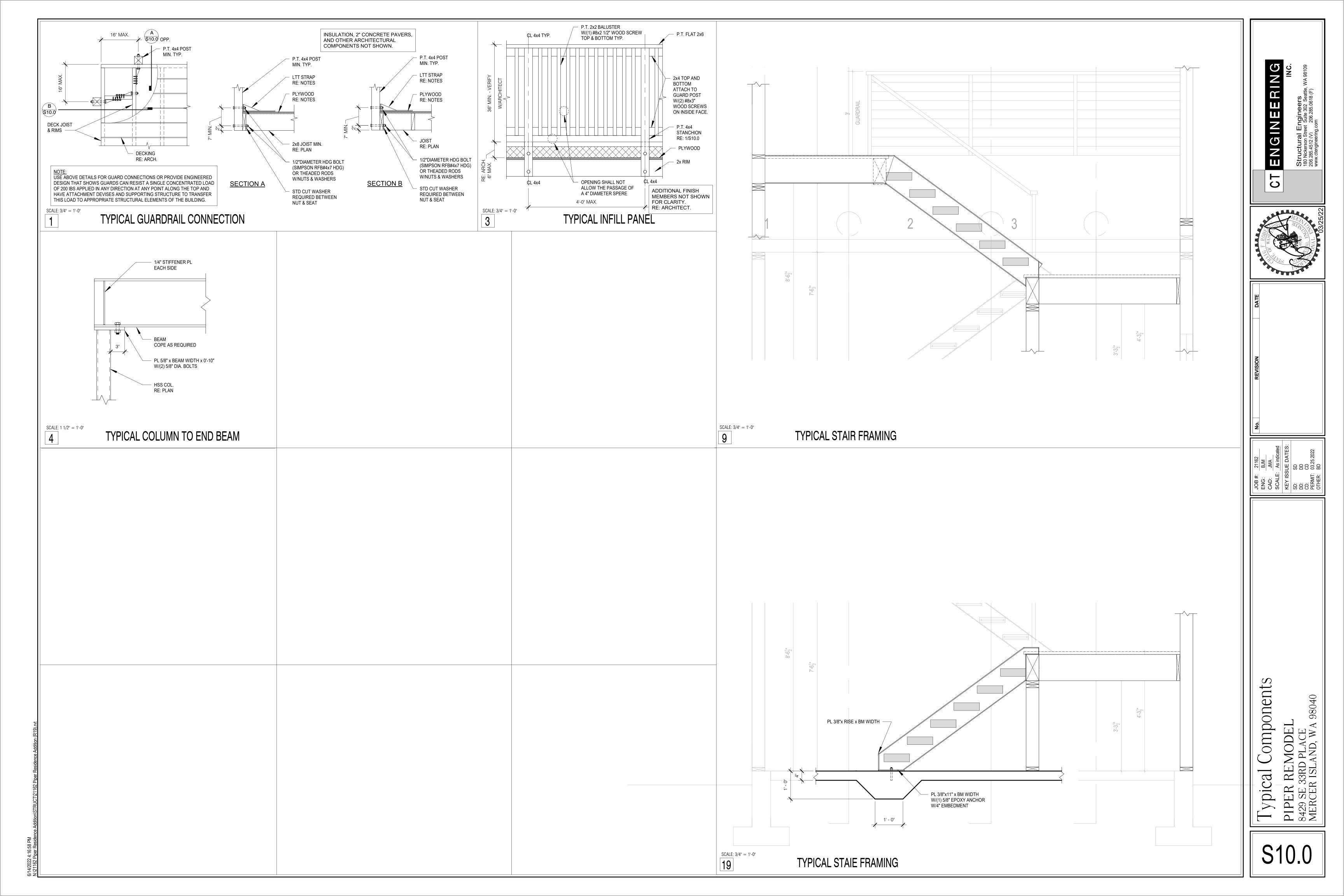
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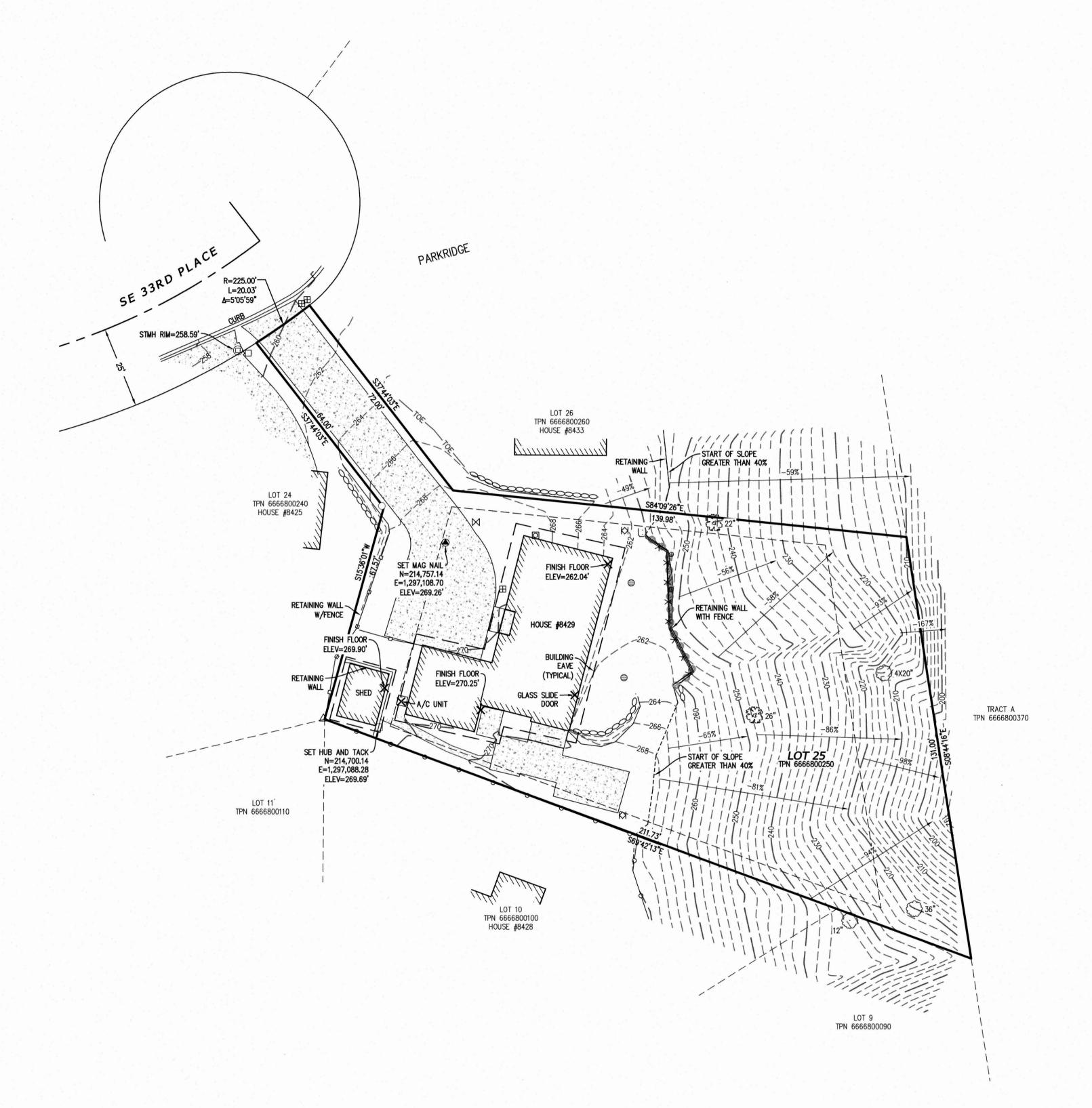






TOPOGRAPHIC MAP

THE NW 1/4 OF THE SW 1/4 OF SECTION 7, TOWNSHIP 24 NORTH, RANGE 5 EAST, W.M.
KING COUNTY, WASHINGTON



LEGAL DESCRIPTION

PER WARRANTY DEED, KING COUNTY RECORDING NO. 20200410000015

LOT 25 OF PARKRIDGE, AS PER PLAT RECORDED IN VOLUME 78 OF PLATS, PAGES 29 AND 30, RECORDS OF KING COUNTY.

SITUATE IN THE CITY OF MERCER ISLAND, COUNTY OF KING, STATE OF WASHINGTON.

HORIZONTAL DATUM

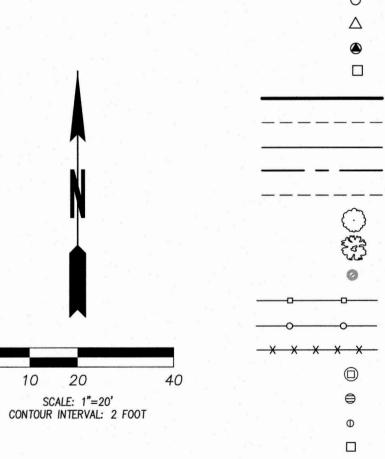
WASHINGTON STATE PLANE COORDINATE SYSTEM, NORTH ZONE (NAD 83/2011) BASED ON RTK GPS MEASUREMENTS CONSTRAINED TO THE WASHINGTON STATE REFERENCE NETWORK.

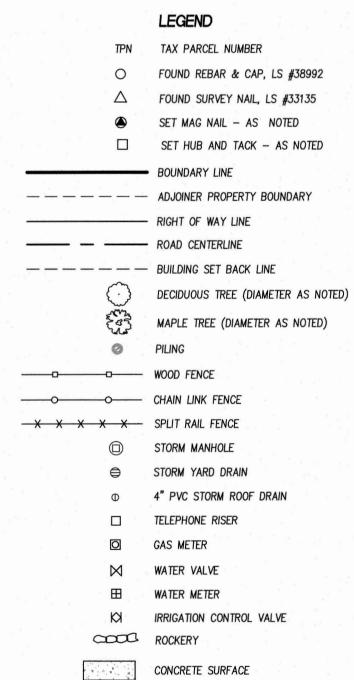
VERTICAL DATUM

NAVD 88 BASED ON RTK GPS MEASUREMENTS CONSTRAINED TO THE WASHINGTON STATE REFERENCE NETWORK.

SURVEY NOTES

- DATA FOR THIS SURVEY WAS GATHERED BY FIELD TRAVERSE UTILIZING ELECTRONIC DATA COLLECTION, AND MEETS OR EXCEEDS
 ACCURACY REQUIREMENTS CONTAINED IN W.A.C. 332.130.090. ALL MEASURING INSTRUMENTS EMPLOYED IN THIS SURVEY HAVE
 BEEN MAINTAINED IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS.
- THIS MAP GRAPHICALLY REPRESENTS CONDITIONS AND FEATURES EXISTING AT THE TIME OF THIS SURVEY ONLY, WHICH WAS PERFORMED DURING <u>DECEMBER OF 2021</u>.
- 3. THE CERTIFICATION OF THIS SURVEY AND MAP IS EXCLUSIVE TO THE NAMED CLIENT WHO REQUESTED THIS SURVEY. IT WAS SPECIFICALLY DESIGNED TO MEET THEIR STATED NEED(S). THAT CERTIFICATION DOES NOT EXTEND TO ANY OTHER PARTIES OR FOR ANY ALTERNATIVE USE OF THIS MAP WITHOUT THE EXPRESS RECERTIFICATION BY THE SURVEYOR NAMING THOSE PARTIES.
- 4. THE PURPOSE OF THIS SURVEY IS TO PROVIDE A TOPOGRAPHIC MAP OF THE EXISTING CONDITIONS WITHIN KING COUNTY PARCEL #6666800250 FOR PLANNING, DESIGN AND CONSTRUCTION.
- 5. UTILITIES OTHER THAN SHOWN MAY EXIST ON THE SITE. THE SURVEYOR DOES NOT WARRANT THAT THE UNDERGROUND UTILITIES SHOWN ARE IN THE EXACT LOCATION INDICATED. LACKING EXCAVATION, THE EXACT LOCATION OF UNDERGROUND FEATURES CANNOT BE ACCURATELY, COMPLETELY, AND RELIABLY DEPICTED. WHERE ADDITIONAL OR MORE DETAILED INFORMATION IS REQUIRED, THE CLIENT IS ADVISED THAT EXCAVATION MAY BE NECESSARY. THE SURVEYOR DOES CERTIFY THAT THEY ARE SHOWN AS ACCURATELY AS POSSIBLE FROM FIELD SURVEY INFORMATION.
- 6. PARCEL AREA: 19,304 ± SQ.FT. (0.44 ACRES)
- 7. ALL DISTANCES AND DIMENSIONS SHOWN ARE U.S. SURVEY FEET GROUND MEASUREMENTS.
- 8. CONTOUR INTERVALS ARE 2-FOOT AND ARE COMPUTER GENERATED FROM GROUND FIELD TOPOGRAPHY GATHERED FOR THIS SURVEY UTILIZING ELECTRONIC DATA COLLECTION.
- 9. THE PROPERTY AND RIGHT-OF-WAY LINES SHOWN HEREON ARE BASED ON FIELD TIES TO SEVERAL OF THE ORIGINAL PLAT MONUMENTS, FROM WHICH WE CONDUCTED A MATHEMATICAL CALCULATION OF THE PARCEL BASED ON THE GEOMETRY OF THE RECORDED PLAT MAP. NO PROPERTY CORNERS WERE ESTABLISHED DURING THIS SURVEY.
- 10. WE HAVE USED GRAPHIC SYMBOLS TO REPRESENT SOME FEATURES ON THIS MAP, SUCH AS UTILITIES, TREES AND FENCES. THE DEFAULT SIZE OF THOSE SYMBOLS MAY NOT REFLECT THE TRUE SIZE OF THE FEATURE THAT WAS MAPPED.





GRAVEL SURFACE

SURVEYOR'S CERTIFICATE

I HEREBY CERTIFY THAT THIS MAP CORRECTLY REPRESENTS A TOPOGRAPHIC SURVEY MADE BY ME OR UNDER MY DIRECTION AND TO THE BEST OF MY KNOWLEDGE REPRESENTS THE TOPOGRAPHIC FEATURES AS THEY EXIST ON THE GROUND AS OF 12/3/2021.

12/20/2021

PROJECT MANAGER

KAP

DESIGN

DRAWN

CHECKED

KAP

SEC 7 T 24 N R

FILE NO 35970

DATE SEALED 12/20/202

CHECKED KAP

SEC 7 T 24 N R 5 E

FILE NO 35970

DATE 12/20/2021

SCALE 1 = 20'

SHEET 1 OF 1

FILE NO 35970

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